JODEC LLC

STEEL DECK CATALOG

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Jodec is a member in good standing of the Steel Deck Institute.

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JODEC LLC

LOAD TABLES AND SECTION PROPERTIES

General Information and Use of Roof Load Table

General

Presented in the Jodec, LLC roof deck load table are allowable uniformly distributed loads.

❖ Steel

Specification - Conforms to ASTM A653 Grade 33; Yield stress 33 ksi and tensile stress of 45 ksi or ASTM A653 Grade 40; Yield stress 40 ksi and tensile stress of 55 ksi or ASTM A653 Grade 50; Yield stress 50 ksi and tensile stress of 65 ksi or ASTM A653 Grade 80; Yield stress 80 ksi and tensile stress of 82 ksi.

Finishes – Painted, G30, G60, or G90. For heavier galvanizing, refer to ASTM A653.

❖ Design Considerations

Strength – Allowable Strength Design (ASD) principles were used in the development of allowable loads in accordance with AISI S100-16, *North American Specification for the Design of Cold Formed Steel Structural Members*. Bending, shear and web crippling are the strength considerations.
The allowable uniformly distributed strength load (S) in the table must be equal to or greater than
(Dead load, DL, + Live load, LL)

Web Crippling – The web crippling strength can vary with the bearing length. The allowable web crippling loads are based on an ending bearing length, N_e= 1.5" and interior bearing length, N_i= 3.0". If the bearing lengths are less than the specified values, the allowable uniformly distributed loads must be checked for the bearing length under consideration (See Example).

Serviceability (Deflection) – The effective moment of inertia for deflection determination is calculated based on an assumed live load stress of $0.6F_y$.
The allowable uniformly distributed deflection load (D) in the table must be equal to or greater than
(Live load, LL)

Example (Use of Load Table)

1.5" ROOF DECK

Given:

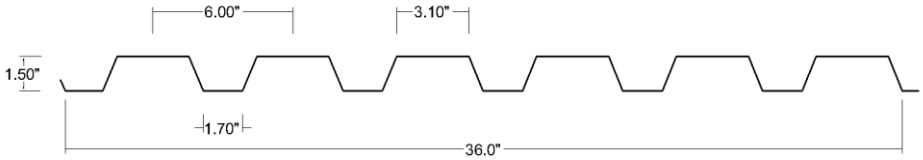
- 3-SPAN continuous, L = 7.5 ft each span
- Deck thickness, t = 0.0295 in, F_y = 40 ksi
- L/180 deflection limit
- End bearing length, N_e = 1 in
- Interior bearing length, N_i = 2 in
- Nominal loads
  1) Dead loads (DL)
     a) deck 2.0 psf
     b) superimposed 10.0 psf
     DL = 12.0 psf
  2) Live load (LL)
     LL = 40.0 psf

Solution:

- ***Strength***
  1) Total load
     [LL + DL]
     [40 + 12.0] = ***52.0 psf***
  2) Allowable load (from Table under "S") is ***63 psf*** (Based on N_e = 1.5 in and N_i = 3.0 in)
     Since ***63 psf*** > ***52.0 psf*** OK
  3) Check end web crippling (N_e = 1.5 in)
     a) End reaction
        $0.400(w)7.5 = P_e$
     b) Allowable end reaction (from Section Property Table)
        $P_e = P_{e1} + P_{e2}\sqrt{N/t}$
        $P_e = 230 + 57.6\sqrt{1/0.0295}$ = 565 lb/ft
        w = 565/0.400/7.5 = ***188 psf***
        Since ***188 psf*** > ***52.0 psf*** OK
  4) Check interior web crippling (N_i = 2 in)
     a) Interior reaction
        $1.10(w)7.5 = P_i$
     b) Allowable interior reaction (from Section Property Table)
        $P_i = P_{i1} + P_{i2}\sqrt{N/t}$
        $P_i = 457 + 77.7\sqrt{2/0.0295}$ = 1097 lb/ft
        w = 1097/1.10/7.5 = ***133 psf***
        Since ***133 psf*** > ***52.0 psf*** OK
- ***Deflection***
  From Table under "D" (L/240) = 48 psf > 40 psf OK
  For L/180, multiply 48 by (240/180) = ***64 psf***
  Since ***64 psf*** > ***40 psf*** OK

1.5 in Roof Deck

Grade 33



SECTION PROPERTIES (Per Foot of Width)

Base Steel Thickness (in)	Weight G90 (psf)	Yield Stress (ksi)	Sec. Modulus		Deflection Moment of Inertia I_{xd} (in^4)	Web Crippling Loads		Web Crippling Data			
			Midspan	Support							
			S_{pos} (in^3)	S_{neg} (in^3)		P_e (lb)	P_i (lb)	P_{e1} End (lb)	P_{e2} End (lb)	P_{i1} Interior (lb)	P_{i2} Interior (lb)
0.0295	1.66	33	0.185	0.184	0.169	529	1024	190	47.5	377	64.1
0.0358	2.01	33	0.231	0.238	0.214	753	1457	288	71.9	570	96.9
0.0474	2.64	33	0.312	0.320	0.290	1256	2427	522	131	1032	175
0.0598	3.31	33	0.397	0.399	0.364	1919	3704	852	213	1681	286

ALLOWABLE UNIFORMLY DISTRIBUTED LOADS (psf)

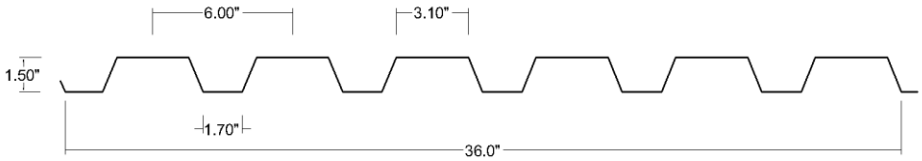
SPAN LENGTH (ft)		1-SPAN				2-SPAN				3-SPAN			
		BASE STEEL THICKNESS (in)				BASE STEEL THICKNESS (in)				BASE STEEL THICKNESS (in)			
		0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598
MAX CO. SPAN (ft-in)		6'1"	7'7"	10'3"	13'1"	7'6"	9'4"	12'8"	16'1"	7'7"	9'6"	12'10"	16'4"
5.0	S	97	122	164	209	97	125	168	210	121	157	211	263
	D	88	112	152	191	211	267	362	455	167	211	287	361
5.5	S	81	101	136	173	80	104	139	174	100	130	174	217
	D	66	84	114	144	158	200	272	342	125	159	216	271
6.0	S	68	85	114	145	67	87	117	146	84	109	146	182
	D	51	65	88	111	122	154	210	263	97	122	166	209
6.5	S	58	72	97	124	57	74	100	124	72	93	125	155
	D	40	51	69	87	96	121	165	207	86	96	131	164
7.0	S	50	62	84	107	50	64	86	107	62	80	107	134
	D	32	41	55	70	77	97	132	166	61	77	105	131
7.5	S	43	54	73	93	43	56	75	93	54	70	94	117
	D	26	33	45	57	62	79	107	135	49	63	85	107
8.0	S	38	48	64	82	38	49	66	82	47	61	82	103
	D	22	27	37	47	51	65	88	111	41	52	70	88
8.5	S	34	42	57	72	34	43	58	73	42	54	73	91
	D	18	23	31	39	43	54	74	93	34	43	58	73
9.0	S	30	38	51	65	30	39	52	65	37	48	65	81
	D	15	19	26	33	36	46	62	78	29	36	49	62
9.5	S	27	34	46	58	27	35	47	58	34	43	58	73
	D	13	16	22	28	31	39	53	66	24	31	42	53
10.0	S	24	30	41	52	24	31	42	53	30	39	53	66
	D	11	14	19	24	26	33	45	57	21	26	36	45
10.5	S	22	28	37	47	22	28	38	48	28	36	48	60
	D	10	12	16	21	23	29	39	49	18	23	31	39
11.0	S	20	25	34	43	20	26	35	43	25	32	44	54
	D	8	11	14	18	20	25	34	43	16	20	27	34
11.5	S	18	23	31	40	18	24	32	40	23	30	40	50
	D	7	9	13	16	17	22	30	37	14	17	24	30
12.0	S	17	21	29	36	17	22	29	36	21	27	37	46
	D	6	8	11	14	15	19	26	33	12	15	21	26

Notes:

1. Based on ASTM A 653 structural steel.
2. Values in row "S" are based on strength.
3. Values in row "D" are based on deflection of SPAN LENGTH/240.
4. P_e = Allowable end web crippling load based on N = 1.5 in.
5. P_i = Allowable interior web crippling load based on N = 3.0 in.
6. If bearing lengths are less than specified, see Example for use of web crippling data.
7. MAX CO. SPAN = Maximum construction span based on 200 lb concentrated load per foot of deck (SDI).
8. Allowable Strength Design principles were used in accordance with AISI S100-16.

1.5 in Roof Deck

Grade 40



SECTION PROPERTIES (Per Foot of Width)											
Base Steel Thickness (in)	Weight G90 (psf)	Yield Stress (ksi)	Sec. Modulus		Deflection Moment of Inertia I_{xd} (in^4)	Web Crippling Loads		Web Crippling Data			
			Midspan	Support				P_{e1} End (lb)	P_{e2} End (lb)	P_{i1} Interior (lb)	P_{i2} Interior (lb)
			S_{pos} (in^3)	S_{neg} (in^3)		P_e (lb)	P_i (lb)				
0.0295	1.66	40	0.177	0.178	0.164	641	1241	230	57.6	457	77.7
0.0358	2.01	40	0.228	0.231	0.209	913	1765	349	87.1	691	117
0.0474	2.64	40	0.309	0.320	0.289	1523	2942	633	158	1251	213
0.0598	3.31	40	0.394	0.399	0.364	2326	4490	1033	258	2037	346

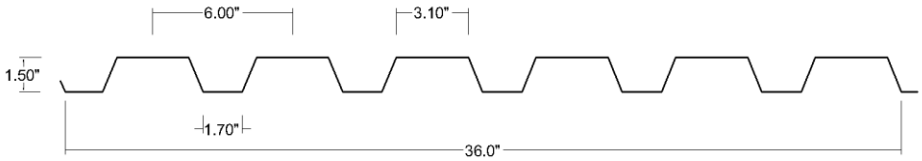
ALLOWABLE UNIFORMLY DISTRIBUTED LOADS (psf)													
SPAN LENGTH (ft)		1-SPAN				2-SPAN				3-SPAN			
		BASE STEEL THICKNESS (in)				BASE STEEL THICKNESS (in)				BASE STEEL THICKNESS (in)			
		0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598
MAX CO. SPAN (ft-in)		7'1"	9'1"	12'4"	15'9"	8'8"	11'3"	15'2"	19'5"	8'10"	11'5"	15'5"	19'8"
5.0	S	113	146	197	252	114	147	204	255	142	184	255	318
	D	86	110	151	191	205	261	361	455	163	207	286	361
5.5	S	93	120	163	208	94	122	169	211	118	152	211	263
	D	65	82	114	144	154	196	271	342	122	156	215	271
6.0	S	78	101	137	175	79	102	142	177	99	128	177	221
	D	50	64	88	111	119	151	209	263	94	120	165	209
6.5	S	67	87	117	149	67	87	121	151	84	109	151	188
	D	39	50	69	87	93	119	164	207	74	94	130	164
7.0	S	58	74	101	129	58	75	104	130	73	94	130	162
	D	31	40	55	70	75	95	131	166	59	75	104	131
7.5	S	50	65	88	112	51	65	91	113	63	82	113	142
	D	26	33	45	57	61	77	107	135	48	61	85	107
8.0	S	44	57	77	98	44	58	80	99	56	72	100	124
	D	21	27	37	47	50	64	88	111	40	51	70	88
8.5	S	39	50	68	87	39	51	71	88	49	64	88	110
	D	18	22	31	39	42	53	73	93	33	42	58	73
9.0	S	35	45	61	78	35	45	63	79	44	57	79	98
	D	15	19	26	33	35	45	62	78	28	36	49	62
9.5	S	31	40	55	70	32	41	57	71	39	51	71	88
	D	13	16	22	28	30	38	53	66	24	30	42	53
10.0	S	28	36	49	63	28	37	51	64	36	46	64	80
	D	11	14	19	24	26	33	45	57	20	26	36	45
10.5	S	26	33	45	57	26	33	46	58	32	42	58	72
	D	9	12	16	21	22	28	39	49	18	22	31	39
11.0	S	23	30	41	52	24	30	42	53	29	38	53	66
	D	8	10	14	18	19	25	34	43	15	19	27	34
11.5	S	21	28	37	48	22	28	39	48	27	35	48	60
	D	7	9	12	16	17	21	30	37	13	17	23	30
12.0	S	20	25	34	44	20	26	35	44	25	32	44	55
	D	6	8	11	14	15	19	26	33	12	15	21	26

Notes:

1. Based on ASTM A 653 structural steel.
2. Values in row "S" are based on strength.
3. Values in row "D" are based on deflection of SPAN LENGTH/240.
4. P_e = Allowable end web crippling load based on N = 1.5 in.
5. P_i = Allowable interior web crippling load based on N = 3.0 in.
6. If bearing lengths are less than specified, see Example for use of web crippling data.
7. MAX CO. SPAN = Maximum construction span based on 200 lb concentrated load per foot of deck (SDI).
8. Allowable Strength Design principles were used in accordance with AISI S100-16.

1.5 in Roof Deck

Grade 50



SECTION PROPERTIES (Per Foot of Width)

Base Steel Thickness (in)	Weight G90 (psf)	Yield Stress (ksi)	Sec. Modulus		Deflection Moment of Inertia I_{xd} (in^4)	Web Crippling Loads		Web Crippling Data			
			Midspan	Support							
			S_{pos} (in^3)	S_{neg} (in^3)		P_e (lb)	P_i (lb)	P_{e1} End (lb)	P_{e2} End (lb)	P_{i1} Interior (lb)	P_{i2} Interior (lb)
0.0295	1.66	50	0.168	0.171	0.159	801	1551	288	72.0	571	97.1
0.0358	2.01	50	0.222	0.222	0.204	1141	2207	436	109	863	147
0.0474	2.64	50	0.305	0.320	0.285	1903	3677	791	198	1563	266
0.0598	3.31	50	0.390	0.399	0.364	2907	5612	1291	323	2546	433

ALLOWABLE UNIFORMLY DISTRIBUTED LOADS (psf)

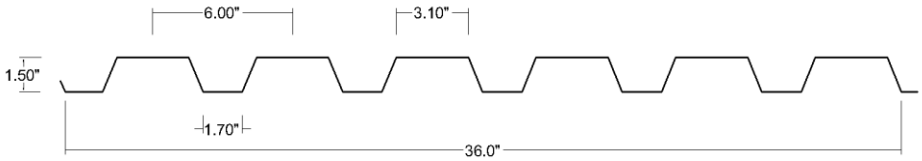
SPAN LENGTH (ft)		1-SPAN				2-SPAN				3-SPAN			
		BASE STEEL THICKNESS (in)				BASE STEEL THICKNESS (in)				BASE STEEL THICKNESS (in)			
		0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598
MAX CO. SPAN (ft-in)		8'4"	11'1"	15'3"	19'6"	10'4"	13'8"	18'9"	24'0"	10'6"	13'10"	19'0"	24'4"
5.0	S	134	177	244	312	137	177	255	318	171	221	319	398
	D	83	107	150	191	198	254	356	455	157	202	282	361
5.5	S	111	147	201	258	113	146	211	263	141	183	264	329
	D	63	80	112	144	149	191	268	342	118	151	212	271
6.0	S	93	123	169	216	95	123	177	221	119	154	222	276
	D	48	62	87	111	115	147	206	263	91	117	163	209
6.5	S	79	105	144	184	81	105	151	188	101	131	189	236
	D	38	49	68	87	90	116	162	207	71	92	128	164
7.0	S	68	90	124	159	70	90	130	162	87	113	163	203
	D	30	39	55	70	72	93	130	166	57	73	103	131
7.5	S	59	79	108	139	61	79	113	142	76	98	142	177
	D	25	32	44	57	59	75	106	135	47	60	84	107
8.0	S	52	69	95	122	53	69	100	124	67	86	125	155
	D	20	26	37	47	48	62	87	111	38	49	69	88
8.5	S	46	61	84	108	47	61	88	110	59	77	110	138
	D	17	22	30	39	40	52	72	93	32	41	57	73
9.0	S	41	55	75	96	42	55	79	98	53	68	98	123
	D	14	18	26	33	34	44	61	78	27	35	48	62
9.5	S	37	49	67	86	38	49	71	88	47	61	88	110
	D	12	16	22	28	29	37	52	66	23	29	41	53
10.0	S	33	44	61	78	34	44	64	80	43	55	80	99
	D	10	13	19	24	25	32	45	57	20	25	35	45
10.5	S	30	40	55	71	31	40	58	72	39	50	72	90
	D	9	12	16	21	21	27	38	49	17	22	30	39
11.0	S	28	37	50	64	28	37	53	66	35	46	66	82
	D	8	10	14	18	19	24	33	43	15	19	27	34
11.5	S	25	34	46	59	26	33	48	60	32	42	60	75
	D	7	9	12	16	16	21	29	37	13	17	23	30
12.0	S	23	31	42	54	24	31	44	55	30	38	55	69
	D	6	8	11	14	14	18	26	33	11	15	20	26

Notes:

1. Based on ASTM A 653 structural steel.
2. Values in row "S" are based on strength.
3. Values in row "D" are based on deflection of SPAN LENGTH/240.
4. P_e = Allowable end web crippling load based on N = 1.5 in.
5. P_i = Allowable interior web crippling load based on N = 3.0 in.
6. If bearing lengths are less than specified, see Example for use of web crippling data.
7. MAX CO. SPAN = Maximum construction span based on 200 lb concentrated load per foot of deck (SDI).
8. Allowable Strength Design principles were used in accordance with AISI S100-16.

1.5 in Roof Deck

Grade 80



SECTION PROPERTIES (Per Foot of Width)											
Base Steel Thickness (in)	Weight G90 (psf)	Yield Stress (ksi)	Sec. Modulus		Deflection Moment of Inertia I_{xd} (in^4)	Web Crippling Loads		Web Crippling Data			
			Midspan	Support				P_{e1} End (lb)	P_{e2} End (lb)	P_{i1} Interior (lb)	P_{i2} Interior (lb)
			S_{pos} (in^3)	S_{neg} (in^3)		P_e (lb)	P_i (lb)				
0.0295	1.66	80	0.161	0.165	0.151	961	1861	345	86.3	686	117
0.0358	2.01	80	0.213	0.215	0.199	1369	2648	523	131	1036	176
0.0474	2.64	80	0.302	0.311	0.281	2284	4413	949	237	1876	319
0.0598	3.31	80	0.387	0.399	0.364	3489	6734	1549	387	3055	519

ALLOWABLE UNIFORMLY DISTRIBUTED LOADS (psf)													
SPAN LENGTH (ft)		1-SPAN				2-SPAN				3-SPAN			
		BASE STEEL THICKNESS (in)				BASE STEEL THICKNESS (in)				BASE STEEL THICKNESS (in)			
		0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598
MAX CO. SPAN (ft-in)		9'8"	12'9"	18'1"	23'2"	11'10"	15'8"	22'3"	28'6"	12'0"	15'11"	22'7"	28'11"
5.0	S	154	204	289	371	158	206	298	382	198	257	372	478
	D	79	104	147	191	189	248	351	454	150	196	278	360
5.5	S	127	168	239	306	131	170	246	316	164	213	308	395
	D	60	78	111	143	142	186	264	341	112	148	209	270
6.0	S	107	142	201	257	110	143	207	265	138	179	259	332
	D	46	60	85	110	109	143	203	263	87	114	161	208
6.5	S	91	121	171	219	94	122	176	226	117	152	220	283
	D	36	47	67	87	86	113	160	207	68	89	127	164
7.0	S	79	104	148	189	81	105	152	195	101	131	190	244
	D	29	38	54	69	69	90	128	165	54	72	101	131
7.5	S	68	91	129	165	70	91	132	170	88	114	166	212
	D	23	31	44	57	56	73	104	135	44	58	82	107
8.0	S	60	80	113	145	62	80	116	149	77	100	145	187
	D	19	25	36	47	46	61	86	111	37	48	68	88
8.5	S	53	71	100	128	55	71	103	132	69	89	129	165
	D	16	21	30	39	38	50	71	92	30	40	57	73
9.0	S	48	63	89	114	49	63	92	118	61	79	115	147
	D	14	18	25	33	32	43	60	78	26	34	48	62
9.5	S	43	56	80	103	44	57	83	106	55	71	103	132
	D	12	15	21	28	28	36	51	66	22	29	41	52
10.0	S	39	51	72	93	40	51	74	96	50	64	93	119
	D	10	13	18	24	24	31	44	57	19	25	35	45
10.5	S	35	46	66	84	36	47	68	87	45	58	84	108
	D	9	11	16	21	20	27	38	49	16	21	30	39
11.0	S	32	42	60	77	33	43	62	79	41	53	77	99
	D	7	10	14	18	18	23	33	43	14	18	26	34
11.5	S	29	39	55	70	30	39	56	72	37	49	70	90
	D	7	9	12	16	16	20	29	37	12	16	23	30
12.0	S	27	35	50	64	28	36	52	66	34	45	65	83
	D	6	8	11	14	14	18	25	33	11	14	20	26

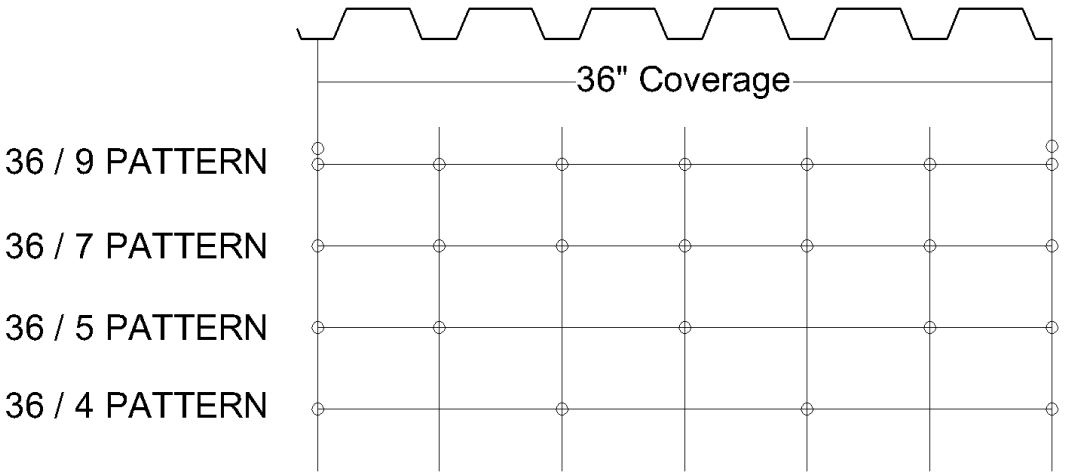
Notes:

1. Based on ASTM A 653 structural steel.
2. Values in row "S" are based on strength.
3. Values in row "D" are based on deflection of SPAN LENGTH/240.
4. P_e = Allowable end web crippling load based on N = 1.5 in.
5. P_i = Allowable interior web crippling load based on N = 3.0 in.
6. If bearing lengths are less than specified, see Example for use of web crippling data.
7. MAX CO. SPAN = Maximum construction span based on 200 lb concentrated load per foot of deck (SDI).
8. Allowable Strength Design principles were used in accordance with AISI S100-16.

JODEC LLC

GENERIC FASTENERS-DIAPHRAGM TABLES

1.5WR Diaphragm Fastening Patterns



Typical Fastener Layout- see first column of diaphragm tables that follow.

1.5WR22

Design thickness = 0.0295 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 45 ksi
Fy: 33 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1775	1560	1390	1230	1100					0.324
	1	1870	1650	1475	1320	1180	1065	970			0.272
	2	1965	1740	1555	1405	1265	1140	1040	950	880	0.234
	3	2055	1825	1635	1480	1345	1215	1110	1015	935	0.206
	4	2145	1910	1715	1555	1420	1290	1175	1080	995	0.183
	5	2225	1990	1790	1625	1485	1365	1245	1145	1055	0.165
	6	2305	2065	1865	1695	1550	1430	1315	1205	1115	0.151
36/7	0	1135	985	855	755	675					0.486
	1	1250	1090	960	850	760	685	625			0.377
	2	1360	1190	1055	940	840	760	695	640	590	0.308
	3	1465	1285	1140	1025	925	835	765	700	650	0.261
	4	1565	1380	1230	1105	1005	910	830	765	705	0.226
	5	1660	1465	1310	1185	1075	985	900	830	765	0.199
	6	1750	1555	1390	1260	1145	1055	970	890	825	0.178
36/5	0	1005	880	785	700	625					0.583
	1	1100	975	870	785	710	640	585			0.433
	2	1190	1060	950	860	785	715	650	600	550	0.345
	3	1275	1140	1025	930	850	785	720	660	610	0.286
	4	1350	1215	1100	1000	915	845	780	725	670	0.245
	5	1420	1285	1165	1065	980	905	840	780	730	0.214
	6	1485	1350	1230	1130	1040	960	895	835	780	0.190
36/4	0	770	675	600	530	470					0.728
	1	860	765	685	620	555	500	455			0.509
	2	950	845	760	690	630	575	525	480	440	0.391
	3	1025	920	835	760	695	640	590	545	500	0.318
	4	1095	990	900	825	760	700	650	605	560	0.267
	5	1155	1055	965	885	815	755	705	660	620	0.231
	6	1210	1110	1020	940	870	810	755	710	665	0.203

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

1.5WR20

Design thickness = 0.0358 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 45 ksi
Fy: 33 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1670	1485	1330							0.357
	1	1780	1605	1440	1300	1185					0.299
	2	1890	1710	1550	1400	1275	1170	1080	1000	935	0.258
	3	2000	1810	1650	1500	1370	1255	1160	1075	1000	0.226
	4	2100	1905	1740	1600	1460	1340	1240	1150	1070	0.202
	5	2200	2000	1830	1685	1550	1425	1315	1225	1140	0.182
	6	2295	2090	1915	1765	1635	1510	1395	1295	1210	0.166
36/7	0	1035	915	820							0.535
	1	1170	1035	930	840	765					0.415
	2	1290	1160	1040	940	860	790	730	675	630	0.340
	3	1405	1265	1150	1040	950	875	805	750	700	0.287
	4	1520	1370	1245	1140	1040	960	885	825	770	0.249
	5	1630	1475	1340	1230	1135	1045	965	900	840	0.219
	6	1735	1570	1435	1315	1215	1130	1045	970	910	0.196
36/5	0	945	845	755							0.642
	1	1055	950	865	785	715					0.477
	2	1160	1050	960	880	805	740	685	635	595	0.380
	3	1265	1145	1050	965	895	825	765	710	660	0.315
	4	1360	1240	1135	1045	970	905	840	785	730	0.270
	5	1445	1325	1220	1125	1045	975	915	855	800	0.236
	6	1530	1405	1295	1200	1120	1045	980	920	870	0.209
36/4	0	725	640	570							0.802
	1	835	755	680	615	560					0.561
	2	935	850	775	715	650	600	550	510	475	0.431
	3	1030	940	865	795	740	685	630	585	545	0.350
	4	1115	1025	945	875	810	760	710	660	615	0.294
	5	1195	1100	1020	945	880	825	775	730	685	0.254
	6	1270	1175	1090	1015	950	890	835	790	745	0.224

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.474 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 45 ksi
Fy: 33 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft. 5	5.5	6	6.5	7	7.5	8	8.5	9	K_1 1/ft
36/9	0	1740									0.410
	1	1910	1725	1575							0.344
	2	2060	1880	1715	1575	1455	1350	1260	1180	1105	0.297
	3	2205	2025	1855	1705	1575	1460	1365	1280	1200	0.261
	4	2340	2150	1990	1835	1695	1575	1470	1375	1295	0.232
	5	2475	2280	2110	1965	1815	1685	1575	1475	1390	0.210
	6	2600	2400	2225	2075	1935	1800	1680	1575	1480	0.191
36/7	0	1070									0.615
	1	1240	1120	1025							0.478
	2	1405	1275	1165	1070	990	920	860	805	760	0.391
	3	1560	1425	1305	1200	1110	1030	965	905	850	0.330
	4	1705	1565	1440	1330	1230	1145	1070	1005	945	0.286
	5	1845	1695	1565	1455	1350	1255	1175	1105	1040	0.253
	6	1985	1825	1690	1570	1465	1370	1280	1200	1130	0.226
36/5	0	990									0.739
	1	1145	1050	955							0.549
	2	1290	1185	1095	1010	930	865	810	760	715	0.437
	3	1420	1310	1215	1130	1055	980	915	855	805	0.363
	4	1550	1430	1330	1240	1160	1090	1020	955	900	0.310
	5	1670	1545	1440	1345	1260	1185	1120	1055	995	0.271
	6	1785	1655	1545	1445	1360	1280	1210	1145	1085	0.241
36/4	0	750									0.923
	1	910	830	755							0.645
	2	1050	965	895	825	760	705	660	615	580	0.496
	3	1175	1085	1010	940	880	820	765	715	675	0.403
	4	1295	1200	1120	1045	980	925	870	815	765	0.339
	5	1405	1305	1220	1145	1075	1015	960	910	860	0.293
	6	1505	1405	1320	1240	1165	1100	1045	990	940	0.257

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft 5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 45 ksi
Fy: 33 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1985									0.387
	2	2180	2005	1855	1720	1605	1505	1415			0.333
	3	2375	2190	2025	1880	1755	1645	1550	1460	1380	0.293
	4	2545	2370	2195	2040	1905	1785	1680	1585	1500	0.261
	5	2710	2525	2365	2200	2055	1925	1815	1710	1620	0.235
	6	2875	2680	2510	2355	2205	2070	1945	1840	1740	0.214
	7	3030	2830	2650	2495	2355	2210	2080	1965	1860	0.197
36/7	1	1295									0.537
	2	1495	1375	1275	1185	1105	1040	975			0.439
	3	1695	1560	1445	1345	1255	1180	1110	1050	995	0.371
	4	1870	1740	1615	1505	1405	1320	1240	1175	1110	0.322
	5	2045	1900	1775	1660	1555	1460	1375	1300	1230	0.284
	6	2215	2060	1925	1810	1705	1600	1510	1425	1350	0.254
	7	2380	2220	2075	1950	1835	1735	1640	1550	1470	0.229
36/5	1	1215									0.617
	2	1395	1295	1200	1120	1045	980	920			0.491
	3	1560	1455	1360	1275	1195	1120	1055	995	945	0.408
	4	1720	1605	1505	1415	1335	1260	1185	1120	1065	0.349
	5	1875	1755	1645	1550	1465	1385	1315	1245	1180	0.304
	6	2020	1895	1780	1680	1585	1505	1430	1360	1300	0.270
	7	2155	2025	1910	1805	1705	1620	1540	1470	1405	0.243
36/4	1	965									0.725
	2	1145	1070	990	920	860	805	755			0.557
	3	1305	1220	1145	1075	1010	945	890	840	795	0.452
	4	1455	1365	1280	1210	1140	1080	1020	965	915	0.381
	5	1595	1500	1410	1335	1260	1200	1140	1085	1035	0.329
	6	1725	1625	1535	1455	1380	1310	1245	1190	1135	0.289
	7	1845	1745	1650	1565	1490	1415	1350	1290	1235	0.258

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	770	675	605	535	475					0.366
	1	865	765	685	620	560	505	460			0.301
	2	950	850	765	695	635	580	530	485	445	0.255
	3	1030	925	835	760	700	645	595	550	505	0.222
	4	1100	995	905	830	760	705	655	610	565	0.196
	5	1160	1060	970	890	820	760	705	660	620	0.176
	6	1215	1115	1025	945	875	815	760	710	665	0.159
36/7	0	490	425	370	330	295					0.549
	1	605	530	470	420	375	340	310			0.414
	2	705	620	555	500	455	415	380	350	320	0.333
	3	795	705	635	575	525	480	445	410	380	0.278
	4	875	785	710	645	590	545	505	470	440	0.239
	5	945	855	775	710	655	605	560	520	490	0.209
	6	1005	915	840	770	710	660	615	575	540	0.186
36/5	0	435	380	340	305	270					0.659
	1	530	470	420	380	350	320	290			0.474
	2	605	545	495	450	415	380	355	330	305	0.370
	3	670	610	560	515	475	440	410	380	360	0.304
	4	720	665	615	570	525	490	460	430	405	0.257
	5	765	710	660	615	575	540	505	475	450	0.223
	6	800	750	700	660	620	580	550	515	490	0.197
36/4	0	335	290	260	230	205					0.823
	1	420	375	340	310	280	260	235			0.554
	2	490	445	405	375	345	320	295	275	260	0.417
	3	545	500	465	430	400	370	345	325	305	0.334
	4	585	545	510	475	445	415	390	370	350	0.279
	5	615	580	545	515	485	460	435	410	390	0.240
	6	640	605	575	545	520	495	470	445	425	0.210

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft. 4	4.5	5	5.5	6	6.5	7	7.5	8	K_1 1/ft
36/9	0	730	650	585							0.403
	1	845	760	695	625	570					0.331
	2	945	860	785	720	665	610	560	520	485	0.281
	3	1045	950	870	805	745	695	640	595	555	0.244
	4	1130	1035	955	880	820	765	715	670	625	0.216
	5	1210	1115	1030	955	890	835	780	735	695	0.193
	6	1285	1190	1100	1025	960	900	845	795	750	0.175
36/7	0	455	400	360							0.605
	1	580	520	470	425	385					0.456
	2	695	625	570	520	480	440	405	380	355	0.366
	3	800	725	660	610	565	525	485	450	425	0.306
	4	895	815	750	690	640	595	560	525	490	0.263
	5	985	900	830	770	715	665	625	585	555	0.230
	6	1060	980	905	840	785	735	690	650	615	0.205
36/5	0	415	370	330							0.726
	1	520	475	430	395	365					0.522
	2	615	565	520	480	445	415	385	360	335	0.408
	3	700	645	595	555	515	480	450	425	400	0.334
	4	770	715	665	620	580	545	515	485	460	0.283
	5	830	775	725	680	640	605	570	540	510	0.246
	6	880	830	780	735	695	655	620	590	560	0.217
36/4	0	315	280	250							0.907
	1	420	385	350	325	295					0.610
	2	510	470	430	400	375	350	325	305	285	0.459
	3	580	540	500	470	440	410	390	365	345	0.368
	4	640	600	560	530	495	470	445	420	400	0.307
	5	685	645	610	580	550	520	495	470	445	0.264
	6	720	685	655	620	590	565	540	515	490	0.231

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft 4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

1.5WR18

Design thickness = 0.0474 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		5	5.5	6	6.5	7	7.5	8	8.5	9	
36/9	0	780									0.464
	1	940	855	780							0.381
	2	1075	990	915	850	785	730	680	635	600	0.324
	3	1205	1115	1035	965	900	840	785	735	690	0.281
	4	1325	1230	1145	1070	1000	940	890	835	785	0.248
	5	1435	1335	1250	1170	1100	1035	980	925	880	0.223
	6	1540	1440	1345	1265	1190	1125	1065	1010	960	0.202
36/7	0	480									0.696
	1	645	585	535							0.525
	2	795	725	670	620	575	535	500	470	440	0.422
	3	930	855	795	740	690	645	605	570	535	0.352
	4	1060	980	910	845	795	745	700	665	630	0.303
	5	1175	1090	1015	950	890	840	795	750	715	0.265
	6	1285	1195	1120	1050	985	930	880	835	795	0.236
36/5	0	445									0.835
	1	590	545	500							0.601
	2	720	665	620	575	540	505	480	450	420	0.469
	3	830	775	725	680	635	600	565	540	510	0.385
	4	930	870	820	770	725	685	650	620	590	0.326
	5	1015	960	905	855	810	765	730	695	660	0.283
	6	1090	1035	980	930	885	840	800	765	730	0.250
36/4	0	335									1.044
	1	480	445	410							0.702
	2	600	560	520	490	460	430	410	385	360	0.528
	3	705	660	620	580	550	520	490	465	445	0.424
	4	785	740	700	665	630	595	570	540	515	0.354
	5	855	810	770	735	700	665	635	610	585	0.304
	6	905	870	830	795	760	730	700	670	645	0.266

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1015									0.428
	2	1195	1110	1035	960	895	840	790			0.363
	3	1355	1265	1185	1115	1045	980	925	870	825	0.316
	4	1510	1410	1325	1245	1180	1115	1055	1000	945	0.279
	5	1650	1550	1460	1375	1300	1235	1175	1115	1065	0.250
	6	1785	1680	1585	1500	1420	1350	1285	1225	1170	0.227
	7	1910	1800	1705	1615	1530	1455	1390	1325	1270	0.207
36/7	1	700									0.590
	2	885	825	765	715	670	625	590			0.474
	3	1055	985	920	865	815	765	725	685	650	0.396
	4	1215	1135	1065	1000	945	895	845	805	765	0.340
	5	1365	1275	1200	1130	1070	1015	965	915	875	0.298
	6	1500	1410	1330	1255	1190	1130	1075	1025	980	0.265
	7	1625	1535	1450	1375	1305	1240	1180	1125	1080	0.239
36/5	1	650									0.675
	2	815	760	715	670	630	600	565			0.527
	3	960	900	845	800	755	715	680	650	620	0.432
	4	1090	1025	970	920	870	830	790	755	720	0.366
	5	1200	1135	1080	1025	975	930	890	850	815	0.318
	6	1300	1235	1180	1125	1075	1025	985	940	905	0.281
	7	1385	1325	1265	1210	1160	1115	1070	1025	985	0.251
36/4	1	540									0.788
	2	690	645	610	575	540	515	490			0.594
	3	820	775	730	695	660	625	595	570	545	0.476
	4	930	885	840	800	760	725	695	665	640	0.397
	5	1020	975	930	890	855	815	785	755	725	0.341
	6	1095	1050	1010	970	935	895	865	830	800	0.299
	7	1155	1115	1075	1040	1000	965	935	900	870	0.266

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: 5/8" arc spot welds of equivalent
Side-lap fastening: #10 screws

Fu: 50 ksi
Fy: 40 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1940	1705	1520	1345	1200					0.324
	1	2045	1805	1610	1445	1290	1165	1060			0.272
	2	2150	1905	1705	1540	1385	1250	1140	1045	960	0.234
	3	2250	2000	1795	1620	1475	1335	1215	1115	1025	0.206
	4	2350	2090	1880	1705	1555	1415	1290	1185	1095	0.183
	5	2440	2180	1965	1780	1630	1500	1370	1255	1160	0.165
	6	2525	2265	2045	1860	1700	1570	1445	1325	1225	0.151
36/7	0	1240	1075	935	825	740					0.486
	1	1365	1190	1050	930	830	750	685			0.377
	2	1490	1300	1155	1030	925	835	760	700	645	0.308
	3	1605	1410	1255	1125	1015	920	840	770	710	0.261
	4	1715	1510	1350	1215	1105	1000	915	840	775	0.226
	5	1820	1610	1440	1300	1185	1085	990	910	845	0.199
	6	1925	1705	1530	1385	1260	1160	1070	980	910	0.178
36/5	0	1095	965	860	765	685					0.583
	1	1205	1065	950	860	775	700	640			0.433
	2	1305	1160	1040	940	860	785	715	655	605	0.345
	3	1395	1250	1125	1020	935	860	790	725	670	0.286
	4	1485	1335	1205	1100	1005	925	860	795	735	0.245
	5	1560	1410	1280	1170	1075	995	920	860	805	0.214
	6	1630	1485	1355	1240	1145	1055	985	920	860	0.190
36/4	0	840	740	655	575	515					0.728
	1	945	835	750	675	605	545	495			0.509
	2	1040	930	835	760	695	630	575	525	485	0.391
	3	1125	1010	915	835	765	705	650	595	550	0.318
	4	1200	1085	990	905	835	770	715	665	615	0.267
	5	1265	1155	1060	975	900	835	775	725	680	0.231
	6	1325	1220	1120	1035	960	890	830	780	730	0.203

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: 5/8" arc spot welds of equivalent
Side-lap fastening: #10 screws

Fu: 50 ksi
Fy: 40 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1855	1650	1475							0.357
	1	1980	1785	1600	1445	1315					0.299
	2	2100	1900	1720	1555	1420	1300	1200	1115	1035	0.258
	3	2220	2010	1830	1670	1520	1395	1290	1195	1115	0.226
	4	2335	2115	1935	1775	1625	1490	1375	1275	1190	0.202
	5	2445	2220	2030	1870	1725	1585	1465	1360	1270	0.182
	6	2555	2325	2130	1960	1815	1680	1550	1440	1345	0.166
36/7	0	1150	1015	910							0.535
	1	1300	1150	1030	935	850					0.415
	2	1435	1285	1155	1045	955	875	810	750	700	0.340
	3	1565	1405	1275	1155	1055	970	895	835	780	0.287
	4	1690	1525	1385	1270	1160	1065	985	915	855	0.249
	5	1810	1635	1490	1365	1260	1160	1075	1000	930	0.219
	6	1930	1745	1595	1465	1350	1255	1160	1080	1010	0.196
36/5	0	1050	940	840							0.642
	1	1175	1060	965	870	795					0.477
	2	1290	1170	1065	980	895	825	760	705	660	0.380
	3	1405	1275	1165	1075	995	920	850	790	735	0.315
	4	1510	1375	1260	1165	1080	1005	935	870	810	0.270
	5	1605	1470	1355	1250	1160	1085	1015	950	890	0.236
	6	1700	1560	1440	1335	1240	1160	1090	1025	965	0.209
36/4	0	805	710	635							0.802
	1	925	835	755	685	620					0.561
	2	1040	945	865	795	725	665	615	570	530	0.431
	3	1145	1045	960	885	820	760	700	650	605	0.350
	4	1240	1140	1050	970	900	840	790	735	685	0.294
	5	1330	1225	1135	1050	980	915	860	810	760	0.254
	6	1410	1305	1210	1130	1055	990	930	875	830	0.224

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: 5/8" arc spot welds of equivalent
Side-lap fastening: #10 screws

Fu: 50 ksi
Fy: 40 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft. 5	5.5	6	6.5	7	7.5	8	8.5	9	K_1 1/ft
36/9	0	1935									0.410
	1	2120	1920	1750							0.344
	2	2290	2090	1905	1750	1615	1500	1400	1310	1230	0.297
	3	2450	2250	2060	1895	1750	1625	1515	1420	1335	0.261
	4	2600	2390	2215	2035	1885	1750	1635	1530	1440	0.232
	5	2750	2530	2345	2180	2015	1875	1750	1640	1545	0.210
	6	2890	2670	2475	2305	2150	2000	1865	1750	1645	0.191
36/7	0	1190									0.615
	1	1375	1245	1135							0.478
	2	1565	1415	1295	1190	1100	1020	955	895	840	0.391
	3	1730	1585	1450	1335	1235	1145	1070	1005	945	0.330
	4	1895	1735	1605	1475	1365	1270	1190	1115	1050	0.286
	5	2050	1885	1740	1615	1500	1395	1305	1225	1155	0.253
	6	2205	2030	1875	1745	1630	1520	1425	1335	1260	0.226
36/5	0	1100									0.739
	1	1275	1165	1065							0.549
	2	1430	1315	1215	1120	1035	965	900	845	790	0.437
	3	1580	1455	1350	1255	1170	1090	1015	955	895	0.363
	4	1720	1590	1475	1375	1290	1210	1135	1065	1000	0.310
	5	1855	1720	1600	1495	1400	1315	1245	1175	1105	0.271
	6	1985	1840	1715	1605	1510	1420	1340	1270	1205	0.241
36/4	0	830									0.923
	1	1015	920	840							0.645
	2	1165	1075	995	915	845	785	730	685	645	0.496
	3	1305	1210	1120	1045	980	910	850	795	750	0.403
	4	1440	1335	1245	1160	1090	1025	965	905	850	0.339
	5	1560	1455	1355	1270	1195	1130	1065	1010	955	0.293
	6	1675	1565	1465	1375	1295	1225	1160	1100	1045	0.257

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft 5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: 5/8" arc spot welds of equivalent
Side-lap fastening: #10 screws

Fu: 50 ksi
Fy: 40 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	2205									0.387
	2	2425	2230	2060	1915	1785	1675	1575			0.333
	3	2640	2430	2250	2090	1950	1830	1720	1625	1535	0.293
	4	2830	2630	2440	2270	2120	1985	1870	1765	1670	0.261
	5	3015	2805	2625	2445	2285	2140	2015	1905	1800	0.235
	6	3195	2980	2790	2620	2450	2300	2165	2040	1935	0.214
	7	3370	3145	2945	2770	2615	2455	2310	2180	2065	0.197
36/7	1	1440									0.537
	2	1660	1530	1415	1315	1230	1155	1085			0.439
	3	1885	1735	1605	1495	1395	1310	1235	1165	1105	0.371
	4	2080	1930	1795	1670	1560	1465	1380	1305	1235	0.322
	5	2275	2115	1975	1845	1725	1620	1530	1445	1370	0.284
	6	2460	2290	2140	2010	1890	1780	1675	1585	1500	0.254
	7	2645	2465	2305	2165	2040	1930	1825	1725	1635	0.229
36/5	1	1350									0.617
	2	1550	1440	1335	1245	1160	1090	1025			0.491
	3	1735	1615	1515	1420	1325	1245	1170	1105	1050	0.408
	4	1915	1785	1675	1575	1485	1400	1320	1245	1180	0.349
	5	2085	1950	1830	1720	1625	1540	1460	1385	1315	0.304
	6	2245	2105	1980	1865	1765	1670	1590	1515	1445	0.270
	7	2395	2250	2120	2005	1895	1800	1710	1630	1560	0.243
36/4	1	1075									0.725
	2	1275	1185	1100	1020	955	895	840			0.557
	3	1455	1355	1270	1195	1120	1050	990	935	885	0.452
	4	1620	1515	1425	1340	1270	1200	1135	1075	1015	0.381
	5	1775	1665	1570	1480	1405	1330	1265	1205	1150	0.329
	6	1920	1810	1705	1615	1530	1455	1385	1320	1265	0.289
	7	2055	1940	1835	1740	1655	1575	1500	1435	1370	0.258

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	855	755	670	595	530					0.366
	1	960	850	765	690	620	560	510			0.301
	2	1060	945	850	770	705	645	585	540	495	0.255
	3	1145	1030	930	845	775	715	665	610	565	0.222
	4	1220	1105	1005	920	845	780	725	675	630	0.196
	5	1290	1175	1075	990	910	845	785	735	690	0.176
	6	1355	1240	1140	1050	975	905	845	790	740	0.159
36/7	0	545	475	415	365	325					0.549
	1	670	585	520	465	420	380	345			0.414
	2	785	690	615	555	505	460	420	385	360	0.333
	3	885	785	705	640	585	535	495	460	425	0.278
	4	970	870	790	715	655	605	560	520	485	0.239
	5	1050	950	865	790	725	670	625	580	545	0.209
	6	1115	1020	935	855	790	735	685	640	600	0.186
36/5	0	485	425	380	335	300					0.659
	1	585	520	470	425	385	355	325			0.474
	2	675	605	550	500	460	425	395	365	340	0.370
	3	745	680	620	570	525	490	455	425	400	0.304
	4	800	740	680	630	585	545	510	480	450	0.257
	5	850	790	735	685	640	600	560	530	500	0.223
	6	885	830	780	730	685	645	610	575	545	0.197
36/4	0	370	325	290	255	225					0.823
	1	470	420	380	345	315	285	260			0.554
	2	545	495	455	415	380	355	330	305	285	0.417
	3	605	555	515	475	445	415	385	360	340	0.334
	4	650	605	565	530	495	465	435	410	390	0.279
	5	680	645	605	570	540	510	480	455	430	0.240
	6	710	675	640	610	575	550	520	495	470	0.210

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	815	725	650							0.403
	1	935	845	770	695	635					0.331
	2	1050	955	875	805	735	675	625	580	540	0.281
	3	1160	1055	970	895	830	770	715	660	620	0.244
	4	1255	1150	1060	980	910	850	795	745	695	0.216
	5	1345	1240	1145	1065	990	925	870	815	770	0.193
	6	1430	1320	1225	1140	1065	1000	940	885	835	0.175
36/7	0	505	445	400							0.605
	1	645	580	520	470	430					0.456
	2	770	695	635	580	535	490	455	420	395	0.366
	3	890	805	735	675	625	580	540	505	470	0.306
	4	995	905	830	770	710	665	620	580	545	0.263
	5	1090	1000	925	855	795	740	695	655	615	0.230
	6	1180	1090	1005	935	870	815	765	720	680	0.205
36/5	0	460	410	370							0.726
	1	580	525	480	440	405					0.522
	2	685	625	575	530	495	460	430	400	375	0.408
	3	780	715	660	615	575	535	505	475	445	0.334
	4	855	795	740	690	645	605	570	540	510	0.283
	5	920	860	805	755	710	670	635	600	570	0.246
	6	980	920	865	820	770	730	690	655	625	0.217
36/4	0	350	310	275							0.907
	1	470	425	390	360	330					0.610
	2	565	520	480	445	415	385	365	340	320	0.459
	3	645	600	560	520	490	460	430	405	385	0.368
	4	710	665	625	585	555	520	495	465	445	0.307
	5	760	720	680	645	610	580	550	520	495	0.264
	6	800	760	725	690	660	625	600	570	545	0.231

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft. 5	5.5	6	6.5	7	7.5	8	8.5	9	K_1 1/ft
36/9	0	865									0.464
	1	1045	955	870							0.381
	2	1195	1100	1020	940	870	810	755	710	665	0.324
	3	1340	1235	1150	1070	1000	935	870	820	770	0.281
	4	1475	1365	1270	1185	1115	1045	990	930	875	0.248
	5	1595	1485	1385	1300	1220	1150	1085	1030	975	0.223
	6	1715	1600	1495	1405	1320	1250	1180	1120	1065	0.202
36/7	0	530									0.696
	1	720	650	595							0.525
	2	880	810	745	690	640	595	555	520	490	0.422
	3	1035	950	880	820	765	720	675	630	595	0.352
	4	1175	1085	1010	940	880	830	780	740	700	0.303
	5	1305	1215	1130	1055	990	935	880	835	790	0.265
	6	1425	1330	1245	1165	1095	1035	980	930	880	0.236
36/5	0	490									0.835
	1	655	605	560							0.601
	2	800	740	685	640	600	565	530	500	470	0.469
	3	925	860	805	755	710	665	630	600	570	0.385
	4	1035	970	910	855	805	765	725	685	655	0.326
	5	1130	1065	1005	950	900	850	810	770	735	0.283
	6	1210	1150	1090	1030	980	935	890	850	810	0.250
36/4	0	370									1.044
	1	535	495	460							0.702
	2	670	620	580	545	510	480	455	425	400	0.528
	3	780	730	685	645	610	575	545	520	495	0.424
	4	875	825	780	735	700	665	630	600	575	0.354
	5	950	900	855	815	775	740	710	675	650	0.304
	6	1010	965	925	885	845	810	775	745	715	0.266

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft 5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1130									0.428
	2	1325	1235	1150	1070	995	935	880			0.363
	3	1505	1405	1315	1235	1165	1090	1025	970	920	0.316
	4	1675	1570	1470	1385	1310	1240	1175	1110	1050	0.279
	5	1835	1720	1620	1530	1445	1370	1305	1240	1185	0.250
	6	1985	1865	1760	1665	1575	1500	1425	1360	1300	0.227
	7	2125	2005	1895	1795	1700	1620	1545	1475	1410	0.207
36/7	1	780									0.590
	2	985	915	855	795	740	695	655			0.474
	3	1175	1090	1020	960	905	855	805	760	720	0.396
	4	1350	1260	1180	1110	1050	990	940	895	855	0.340
	5	1515	1420	1335	1255	1190	1125	1070	1020	970	0.298
	6	1670	1565	1475	1395	1320	1255	1195	1140	1085	0.265
	7	1810	1705	1610	1525	1450	1375	1310	1250	1200	0.239
36/5	1	725									0.675
	2	905	845	790	745	705	665	630			0.527
	3	1065	1000	940	890	840	795	760	720	690	0.432
	4	1210	1140	1075	1020	970	920	875	840	800	0.366
	5	1335	1265	1200	1140	1085	1035	990	945	905	0.318
	6	1445	1375	1310	1250	1195	1140	1090	1045	1005	0.281
	7	1540	1470	1405	1345	1290	1235	1185	1140	1095	0.251
36/4	1	600									0.788
	2	770	720	675	635	605	570	545			0.594
	3	910	860	815	770	730	695	665	635	605	0.476
	4	1035	980	935	890	845	810	775	740	710	0.397
	5	1135	1085	1035	990	950	910	870	835	805	0.341
	6	1220	1170	1125	1080	1035	995	960	925	890	0.299
	7	1285	1240	1195	1155	1115	1075	1035	1000	970	0.266

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 60 ksi
Fy: 50 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	2150	1890	1685	1490	1330					0.324
	1	2280	2015	1795	1615	1440	1300	1185			0.272
	2	2405	2130	1905	1720	1550	1400	1275	1170	1080	0.234
	3	2525	2245	2010	1820	1660	1500	1370	1255	1155	0.206
	4	2635	2350	2115	1920	1750	1600	1460	1340	1235	0.183
	5	2745	2455	2215	2010	1840	1695	1550	1425	1315	0.165
	6	2845	2555	2310	2105	1925	1775	1645	1510	1395	0.151
36/7	0	1375	1195	1040	920	820					0.486
	1	1525	1330	1175	1040	930	840	765			0.377
	2	1670	1465	1300	1165	1040	940	860	790	730	0.308
	3	1810	1590	1415	1275	1150	1040	950	875	805	0.261
	4	1945	1715	1530	1380	1255	1140	1045	960	885	0.226
	5	2070	1830	1640	1480	1350	1235	1135	1045	965	0.199
	6	2185	1945	1745	1580	1440	1325	1220	1130	1045	0.178
36/5	0	1215	1070	950	850	760					0.583
	1	1345	1190	1065	960	870	785	715			0.433
	2	1465	1305	1170	1060	965	885	805	740	685	0.345
	3	1575	1410	1270	1155	1055	970	900	825	765	0.286
	4	1675	1505	1365	1245	1140	1055	975	910	840	0.245
	5	1765	1600	1455	1330	1225	1130	1050	980	920	0.214
	6	1845	1680	1540	1410	1305	1205	1125	1050	985	0.190
36/4	0	930	820	725	640	570					0.728
	1	1055	935	840	760	680	615	560			0.509
	2	1170	1045	940	855	780	715	650	595	550	0.391
	3	1270	1145	1035	945	870	800	740	680	630	0.318
	4	1355	1230	1125	1030	950	880	815	760	710	0.267
	5	1435	1310	1205	1110	1025	950	885	830	780	0.231
	6	1500	1385	1275	1180	1095	1020	955	895	840	0.203

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 60 ksi
Fy: 50 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	2225	1985	1770							0.357
	1	2375	2140	1920	1735	1580					0.299
	2	2525	2280	2065	1870	1700	1560	1440	1335	1245	0.258
	3	2665	2410	2200	2000	1825	1675	1545	1435	1335	0.226
	4	2800	2540	2320	2135	1950	1790	1650	1535	1430	0.202
	5	2935	2665	2440	2245	2070	1900	1755	1630	1520	0.182
	6	3065	2790	2555	2355	2180	2015	1860	1730	1615	0.166
36/7	0	1380	1220	1090							0.535
	1	1560	1385	1240	1120	1020					0.415
	2	1720	1545	1385	1255	1145	1050	970	905	840	0.340
	3	1875	1690	1535	1390	1265	1165	1075	1000	935	0.287
	4	2030	1830	1665	1525	1390	1280	1185	1100	1025	0.249
	5	2175	1965	1790	1640	1515	1390	1290	1200	1120	0.219
	6	2315	2095	1915	1755	1625	1505	1395	1295	1210	0.196
36/5	0	1260	1130	1010							0.642
	1	1410	1270	1155	1045	955					0.477
	2	1550	1405	1280	1175	1075	990	915	850	790	0.380
	3	1685	1530	1400	1290	1195	1100	1020	945	885	0.315
	4	1810	1650	1515	1395	1295	1205	1125	1045	975	0.270
	5	1930	1765	1625	1500	1395	1300	1220	1145	1065	0.236
	6	2040	1875	1730	1600	1490	1395	1305	1230	1160	0.209
36/4	0	965	855	760							0.802
	1	1110	1005	910	820	745					0.561
	2	1250	1135	1035	955	870	800	735	685	635	0.431
	3	1375	1255	1150	1060	985	910	840	780	730	0.350
	4	1490	1365	1260	1165	1085	1010	945	880	820	0.294
	5	1595	1470	1360	1265	1175	1100	1035	975	915	0.254
	6	1695	1565	1455	1355	1265	1185	1115	1055	995	0.224

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 60 ksi
Fy: 50 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		5	5.5	6	6.5	7	7.5	8	8.5	9	
36/9	0	2320									0.410
	1	2545	2300	2100							0.344
	2	2750	2505	2285	2100	1940	1800	1680	1570	1475	0.297
	3	2940	2700	2475	2270	2100	1950	1820	1705	1600	0.261
	4	3120	2870	2655	2445	2260	2100	1960	1835	1725	0.232
	5	3300	3040	2815	2620	2420	2250	2100	1970	1850	0.210
	6	3470	3200	2970	2765	2580	2400	2240	2100	1975	0.191
36/7	0	1425									0.615
	1	1650	1495	1365							0.478
	2	1875	1700	1550	1425	1320	1225	1145	1075	1010	0.391
	3	2080	1905	1740	1600	1480	1375	1285	1205	1135	0.330
	4	2275	2085	1925	1775	1640	1525	1425	1340	1260	0.286
	5	2465	2260	2090	1940	1800	1675	1570	1470	1385	0.253
	6	2645	2435	2250	2095	1955	1825	1710	1605	1510	0.226
36/5	0	1320									0.739
	1	1530	1400	1275							0.549
	2	1720	1580	1460	1345	1245	1155	1080	1010	950	0.437
	3	1895	1750	1620	1505	1405	1305	1220	1145	1075	0.363
	4	2065	1910	1770	1655	1545	1455	1360	1275	1200	0.310
	5	2230	2065	1920	1795	1680	1580	1490	1410	1325	0.271
	6	2380	2210	2060	1930	1810	1705	1610	1525	1450	0.241
36/4	0	1000									0.923
	1	1215	1105	1010							0.645
	2	1400	1290	1195	1100	1015	940	880	825	775	0.496
	3	1570	1450	1345	1255	1175	1090	1020	955	900	0.403
	4	1725	1600	1490	1395	1310	1230	1160	1085	1025	0.339
	5	1875	1745	1630	1525	1435	1355	1280	1215	1150	0.293
	6	2010	1875	1760	1650	1555	1470	1390	1320	1255	0.257

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

1.5WR16

Design thickness = 0.0598 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 60 ksi
Fy: 50 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	2490									0.387
	2	2760	2535	2345	2175	2030	1905	1790			0.333
	3	3010	2780	2570	2390	2230	2090	1965	1855	1755	0.293
	4	3235	3010	2800	2600	2430	2280	2145	2025	1915	0.261
	5	3455	3220	3010	2815	2630	2465	2320	2190	2075	0.235
	6	3670	3425	3205	3015	2830	2655	2500	2360	2235	0.214
	7	3875	3620	3395	3195	3015	2840	2675	2525	2395	0.197
36/7	1	1635									0.537
	2	1900	1750	1620	1505	1405	1320	1245			0.439
	3	2160	1995	1845	1720	1605	1505	1420	1340	1270	0.371
	4	2395	2225	2075	1930	1805	1695	1595	1510	1430	0.322
	5	2625	2445	2280	2140	2005	1880	1775	1675	1590	0.284
	6	2850	2655	2480	2330	2195	2070	1950	1845	1750	0.254
	7	3070	2860	2680	2515	2370	2240	2125	2010	1905	0.229
36/5	1	1530									0.617
	2	1770	1645	1530	1425	1330	1245	1175			0.491
	3	1990	1855	1735	1630	1530	1435	1350	1275	1210	0.408
	4	2205	2055	1930	1815	1710	1620	1530	1445	1370	0.349
	5	2405	2250	2110	1990	1880	1780	1690	1610	1525	0.304
	6	2590	2430	2290	2160	2045	1940	1840	1755	1675	0.270
	7	2770	2605	2455	2320	2200	2090	1990	1895	1810	0.243
36/4	1	1225									0.725
	2	1460	1360	1265	1175	1100	1030	970			0.557
	3	1670	1560	1465	1375	1295	1215	1145	1080	1025	0.452
	4	1870	1750	1645	1550	1465	1390	1320	1250	1180	0.381
	5	2050	1930	1815	1715	1625	1545	1470	1400	1340	0.329
	6	2220	2090	1975	1875	1775	1690	1610	1540	1470	0.289
	7	2375	2245	2125	2020	1920	1830	1745	1670	1600	0.258

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1030	905	805	710	635					0.366
	1	1155	1025	915	825	745	675	615			0.301
	2	1270	1135	1020	925	845	775	705	645	595	0.255
	3	1375	1235	1115	1015	930	860	795	730	675	0.222
	4	1465	1330	1210	1105	1015	940	870	810	755	0.196
	5	1550	1410	1290	1185	1095	1015	945	880	825	0.176
	6	1625	1490	1370	1260	1170	1085	1010	945	890	0.159
36/7	0	655	570	495	440	390					0.549
	1	805	705	625	560	500	455	415			0.414
	2	940	830	740	665	605	555	505	465	430	0.333
	3	1060	945	845	765	700	645	595	550	510	0.278
	4	1165	1045	945	860	790	725	675	625	585	0.239
	5	1260	1140	1035	950	870	805	750	695	655	0.209
	6	1340	1225	1120	1030	950	880	820	765	720	0.186
36/5	0	580	510	455	405	360					0.659
	1	705	625	565	510	465	425	390			0.474
	2	810	730	660	600	550	510	475	440	410	0.370
	3	895	815	745	685	630	585	545	510	480	0.304
	4	960	885	820	760	705	655	615	575	540	0.257
	5	1020	950	885	825	770	720	675	635	600	0.223
	6	1065	1000	935	880	825	775	730	690	655	0.197
36/4	0	445	390	345	305	270					0.823
	1	565	505	455	410	375	345	315			0.554
	2	655	595	545	500	460	425	395	370	345	0.417
	3	725	670	620	575	530	495	465	435	410	0.334
	4	780	725	680	635	595	555	525	495	465	0.279
	5	820	775	730	685	650	610	580	545	520	0.240
	6	850	810	770	730	695	660	625	595	565	0.210

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	975	870	780							0.403
	1	1125	1015	925	835	760					0.331
	2	1265	1145	1050	965	885	810	750	695	650	0.281
	3	1390	1270	1165	1075	995	925	855	795	740	0.244
	4	1510	1380	1270	1175	1095	1020	955	895	835	0.216
	5	1615	1490	1375	1275	1190	1110	1040	980	925	0.193
	6	1715	1585	1470	1370	1280	1200	1125	1060	1005	0.175
36/7	0	605	535	480							0.605
	1	775	695	625	565	515					0.456
	2	925	835	760	695	640	590	545	505	470	0.366
	3	1065	965	885	810	750	700	650	605	565	0.306
	4	1195	1090	1000	920	855	795	745	700	655	0.263
	5	1310	1200	1110	1025	955	890	835	785	740	0.230
	6	1415	1305	1210	1120	1045	980	920	865	820	0.205
36/5	0	550	495	440							0.726
	1	695	630	575	530	485					0.522
	2	825	755	690	640	590	550	515	480	450	0.408
	3	935	860	795	740	690	645	605	570	535	0.334
	4	1025	955	885	830	775	730	685	645	610	0.283
	5	1105	1035	970	910	855	805	760	720	685	0.246
	6	1175	1105	1040	980	925	875	830	790	750	0.217
36/4	0	425	375	335							0.907
	1	565	510	470	430	395					0.610
	2	680	625	575	535	500	465	435	410	380	0.459
	3	775	720	670	625	585	550	520	490	465	0.368
	4	850	800	750	705	665	625	590	560	535	0.307
	5	910	865	815	770	730	695	660	625	595	0.264
	6	960	915	870	830	790	755	720	685	655	0.231

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft. 5	5.5	6	6.5	7	7.5	8	8.5	9	K_1 1/ft
36/9	0	1040									0.464
	1	1250	1145	1045							0.381
	2	1435	1320	1225	1130	1045	970	905	850	800	0.324
	3	1605	1485	1380	1285	1200	1120	1045	980	925	0.281
	4	1770	1640	1525	1425	1335	1255	1185	1115	1050	0.248
	5	1915	1785	1665	1560	1465	1380	1305	1235	1175	0.223
	6	2055	1920	1795	1685	1585	1500	1420	1345	1280	0.202
36/7	0	640									0.696
	1	860	780	715							0.525
	2	1060	970	895	830	770	715	670	625	590	0.422
	3	1240	1145	1060	985	920	865	810	760	715	0.352
	4	1410	1305	1210	1130	1060	995	935	885	840	0.303
	5	1570	1455	1355	1270	1190	1120	1060	1000	950	0.265
	6	1710	1595	1490	1400	1315	1240	1175	1115	1060	0.236
36/5	0	590									0.835
	1	790	725	670							0.601
	2	960	885	825	770	720	675	640	600	565	0.469
	3	1110	1035	965	905	850	800	755	720	680	0.385
	4	1245	1165	1090	1030	970	915	870	825	785	0.326
	5	1355	1280	1205	1140	1080	1025	975	925	885	0.283
	6	1455	1380	1305	1240	1180	1120	1070	1020	975	0.250
36/4	0	445									1.044
	1	645	595	550							0.702
	2	805	745	695	650	610	575	545	515	485	0.528
	3	940	880	825	775	735	695	655	625	595	0.424
	4	1050	990	935	885	840	795	760	725	690	0.354
	5	1140	1080	1030	980	935	890	850	815	780	0.304
	6	1210	1160	1110	1060	1015	970	930	895	860	0.266

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft 5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

1.5WR16

Design thickness = 0.0598 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	2.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1355									0.428
	2	1590	1480	1380	1280	1195	1120	1055			0.363
	3	1810	1685	1580	1485	1395	1310	1235	1165	1100	0.316
	4	2015	1885	1765	1665	1570	1490	1410	1330	1260	0.279
	5	2205	2065	1945	1835	1735	1645	1565	1490	1420	0.250
	6	2385	2240	2115	2000	1895	1800	1710	1630	1560	0.227
	7	2550	2405	2270	2150	2045	1945	1850	1770	1690	0.207
36/7	1	935									0.590
	2	1180	1100	1025	955	890	835	790			0.474
	3	1410	1310	1225	1150	1085	1025	965	910	865	0.396
	4	1620	1515	1420	1335	1260	1190	1130	1075	1025	0.340
	5	1820	1705	1600	1510	1425	1350	1285	1225	1165	0.298
	6	2000	1880	1775	1675	1585	1505	1435	1365	1305	0.265
	7	2170	2045	1935	1830	1740	1655	1575	1505	1440	0.239
36/5	1	870									0.675
	2	1085	1015	950	895	845	800	755			0.527
	3	1280	1200	1130	1065	1010	955	910	865	830	0.432
	4	1450	1370	1295	1225	1160	1105	1055	1005	960	0.366
	5	1600	1515	1440	1370	1305	1245	1185	1135	1090	0.318
	6	1730	1650	1570	1500	1430	1370	1310	1255	1205	0.281
	7	1845	1765	1690	1615	1550	1485	1425	1370	1315	0.251
36/4	1	720									0.788
	2	920	865	810	765	725	685	650			0.594
	3	1095	1035	975	925	880	835	795	760	725	0.476
	4	1240	1180	1120	1065	1015	970	930	890	850	0.397
	5	1360	1300	1245	1190	1140	1090	1045	1005	965	0.341
	6	1460	1405	1350	1295	1245	1195	1150	1110	1070	0.299
	7	1545	1490	1435	1385	1335	1290	1245	1205	1165	0.266

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Grade 80

1.5WR22

Design thickness = 0.0295 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 62 ksi
Fy: 60 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	2190	1925	1715	1520	1355					0.324
	1	2325	2050	1830	1645	1470	1325	1210			0.272
	2	2455	2175	1945	1755	1585	1430	1305	1195	1100	0.234
	3	2575	2290	2055	1860	1695	1535	1400	1280	1185	0.206
	4	2695	2405	2160	1960	1790	1640	1495	1370	1265	0.183
	5	2805	2510	2265	2055	1880	1730	1590	1455	1345	0.165
	6	2910	2615	2365	2150	1970	1815	1685	1545	1425	0.151
36/7	0	1400	1215	1060	935	835					0.486
	1	1560	1360	1200	1060	950	860	785			0.377
	2	1710	1495	1325	1190	1065	960	880	805	745	0.308
	3	1850	1625	1450	1300	1180	1065	975	895	825	0.261
	4	1985	1755	1565	1410	1285	1170	1070	980	905	0.226
	5	2115	1875	1680	1515	1380	1265	1165	1070	990	0.199
	6	2235	1990	1785	1620	1475	1355	1255	1155	1070	0.178
36/5	0	1240	1090	970	865	770					0.583
	1	1375	1215	1085	980	885	800	730			0.433
	2	1495	1330	1195	1080	985	905	825	755	700	0.345
	3	1610	1440	1300	1180	1080	995	920	845	780	0.286
	4	1710	1540	1395	1275	1170	1080	1000	930	860	0.245
	5	1805	1635	1490	1360	1255	1160	1075	1005	940	0.214
	6	1890	1720	1575	1445	1335	1235	1150	1075	1005	0.190
36/4	0	950	835	740	650	580					0.728
	1	1080	955	855	775	695	625	570			0.509
	2	1195	1070	965	875	800	730	665	610	565	0.391
	3	1295	1170	1060	965	890	820	760	700	645	0.318
	4	1385	1260	1150	1055	970	900	835	780	725	0.267
	5	1465	1340	1230	1135	1050	975	910	850	795	0.231
	6	1535	1415	1305	1210	1120	1045	975	915	860	0.203

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Grade 80

1.5WR20

Design thickness = 0.0358 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 62 ksi
Fy: 60 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	2300	2050	1830							0.357
	1	2455	2215	1985	1790	1630					0.299
	2	2605	2355	2135	1930	1760	1615	1490	1380	1285	0.258
	3	2755	2490	2270	2070	1885	1730	1600	1485	1380	0.226
	4	2895	2625	2400	2205	2015	1850	1705	1585	1475	0.202
	5	3035	2755	2520	2320	2140	1965	1815	1685	1575	0.182
	6	3165	2880	2640	2435	2255	2085	1925	1790	1670	0.166
36/7	0	1425	1260	1130							0.535
	1	1610	1430	1280	1160	1055					0.415
	2	1780	1595	1435	1295	1185	1085	1005	935	870	0.340
	3	1940	1745	1585	1435	1310	1205	1115	1035	965	0.287
	4	2095	1890	1720	1575	1435	1320	1220	1135	1060	0.249
	5	2245	2030	1850	1695	1565	1440	1330	1240	1155	0.219
	6	2395	2165	1975	1815	1680	1555	1440	1340	1250	0.196
36/5	0	1300	1165	1040							0.642
	1	1455	1315	1195	1080	985					0.477
	2	1600	1450	1325	1215	1110	1020	945	875	815	0.380
	3	1740	1580	1445	1330	1235	1140	1050	980	915	0.315
	4	1870	1705	1565	1445	1340	1245	1160	1080	1010	0.270
	5	1995	1825	1680	1550	1440	1345	1260	1180	1105	0.236
	6	2110	1935	1785	1655	1540	1440	1350	1270	1200	0.209
36/4	0	995	880	785							0.802
	1	1150	1040	940	850	770					0.561
	2	1290	1170	1070	985	900	825	760	705	660	0.431
	3	1420	1295	1190	1100	1020	940	870	805	755	0.350
	4	1540	1410	1300	1205	1120	1045	980	910	850	0.294
	5	1650	1520	1405	1305	1215	1140	1070	1005	945	0.254
	6	1750	1620	1505	1400	1310	1225	1155	1090	1030	0.224

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Grade 80

1.5WR18

Design thickness = 0.0474 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 62 ksi
Fy: 60 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft. 5	5.5	6	6.5	7	7.5	8	8.5	9	K_1 1/ft
36/9	0	2400									0.410
	1	2630	2380	2170							0.344
	2	2840	2590	2360	2170	2005	1860	1735	1625	1525	0.297
	3	3035	2790	2555	2350	2170	2015	1880	1760	1655	0.261
	4	3225	2965	2745	2525	2335	2170	2025	1900	1785	0.232
	5	3410	3140	2910	2705	2500	2325	2170	2035	1915	0.210
	6	3585	3310	3070	2860	2670	2480	2315	2170	2045	0.191
36/7	0	1475									0.615
	1	1705	1545	1410							0.478
	2	1940	1755	1605	1475	1365	1270	1185	1110	1045	0.391
	3	2150	1965	1800	1655	1530	1425	1330	1245	1175	0.330
	4	2350	2155	1990	1830	1695	1580	1475	1385	1305	0.286
	5	2545	2335	2160	2005	1860	1735	1620	1520	1430	0.253
	6	2735	2515	2325	2165	2020	1890	1765	1655	1560	0.226
36/5	0	1365									0.739
	1	1580	1445	1320							0.549
	2	1775	1630	1510	1390	1285	1195	1115	1045	985	0.437
	3	1960	1805	1675	1555	1450	1350	1260	1180	1110	0.363
	4	2135	1975	1830	1710	1600	1500	1405	1320	1240	0.310
	5	2300	2130	1985	1855	1735	1635	1540	1455	1370	0.271
	6	2460	2285	2130	1995	1870	1765	1665	1575	1500	0.241
36/4	0	1035									0.923
	1	1260	1145	1040							0.645
	2	1445	1330	1235	1135	1050	975	910	850	800	0.496
	3	1620	1500	1390	1300	1215	1130	1055	985	930	0.403
	4	1785	1655	1540	1440	1350	1275	1200	1125	1055	0.339
	5	1935	1800	1685	1580	1485	1400	1325	1255	1185	0.293
	6	2075	1940	1815	1705	1610	1520	1440	1365	1300	0.257

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft 5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Grade 80

1.5WR16

Design thickness = 0.0598 in
Support fastening: 5/8" arc spot welds or equivalent
Side-lap fastening: #10 screws

Fu: 62 ksi
Fy: 60 ksi
Fxx: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.55	3.00
Wind	0.75	2.15
Other	0.55	3.00

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	2500									0.387
	2	2775	2550	2360	2190	2045	1915	1800			0.333
	3	3035	2805	2595	2410	2250	2110	1985	1875	1770	0.293
	4	3265	3040	2830	2630	2455	2305	2170	2045	1935	0.261
	5	3490	3255	3045	2850	2660	2495	2350	2220	2100	0.235
	6	3710	3465	3245	3050	2870	2690	2535	2390	2265	0.214
	7	3925	3665	3440	3235	3055	2885	2715	2565	2430	0.197
36/7	1	1645									0.537
	2	1920	1765	1635	1520	1420	1335	1255			0.439
	3	2185	2020	1870	1740	1625	1525	1435	1360	1285	0.371
	4	2430	2255	2105	1960	1830	1720	1620	1530	1450	0.322
	5	2665	2480	2315	2170	2035	1915	1805	1705	1615	0.284
	6	2895	2695	2520	2365	2230	2105	1985	1875	1780	0.254
	7	3115	2910	2725	2560	2410	2280	2165	2050	1945	0.229
36/5	1	1540									0.617
	2	1785	1660	1545	1435	1345	1260	1185			0.491
	3	2015	1875	1755	1650	1550	1455	1370	1295	1225	0.408
	4	2230	2085	1955	1840	1735	1640	1550	1465	1390	0.349
	5	2435	2280	2140	2020	1910	1810	1715	1635	1555	0.304
	6	2630	2470	2325	2195	2075	1970	1870	1785	1705	0.270
	7	2810	2645	2495	2360	2235	2125	2020	1930	1845	0.243
36/4	1	1230									0.725
	2	1475	1375	1280	1190	1110	1040	980			0.557
	3	1690	1580	1485	1395	1315	1235	1160	1095	1040	0.452
	4	1895	1775	1670	1575	1490	1410	1340	1270	1205	0.381
	5	2080	1955	1845	1745	1650	1570	1495	1425	1360	0.329
	6	2250	2125	2010	1905	1805	1720	1640	1565	1495	0.289
	7	2405	2280	2160	2055	1955	1860	1775	1700	1630	0.258

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	3.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1060	935	830	735	655					0.366
	1	1195	1060	945	855	770	695	635			0.301
	2	1315	1170	1055	955	875	800	730	670	615	0.255
	3	1420	1275	1155	1050	965	890	825	755	700	0.222
	4	1515	1370	1250	1140	1050	970	900	840	780	0.196
	5	1600	1460	1335	1225	1130	1050	975	910	855	0.176
	6	1680	1540	1415	1305	1205	1120	1045	980	920	0.159
36/7	0	680	590	515	455	405					0.549
	1	830	730	645	580	520	470	430			0.414
	2	970	855	765	690	625	575	525	480	445	0.333
	3	1095	975	875	795	725	665	615	570	525	0.278
	4	1205	1080	980	890	815	750	695	645	605	0.239
	5	1300	1180	1070	980	900	835	775	720	675	0.209
	6	1385	1265	1155	1065	980	910	845	790	740	0.186
36/5	0	600	530	470	420	375					0.659
	1	730	650	580	525	480	440	400			0.474
	2	835	755	680	620	570	525	490	455	420	0.370
	3	925	840	770	710	655	605	565	525	495	0.304
	4	995	915	845	785	725	675	635	595	560	0.257
	5	1050	980	910	850	795	745	695	655	620	0.223
	6	1100	1035	970	910	855	800	755	715	675	0.197
36/4	0	460	405	360	315	280					0.823
	1	580	520	470	425	390	355	325			0.554
	2	675	615	560	515	475	440	410	380	355	0.417
	3	750	690	640	590	550	510	480	450	420	0.334
	4	805	750	700	655	615	575	540	510	480	0.279
	5	845	800	755	710	670	630	595	565	535	0.240
	6	880	840	795	755	715	680	645	615	585	0.210

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Grade 80

1.5WR20

Design thickness = 0.0358 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	3.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1010	900	805							0.403
	1	1165	1050	955	865	785					0.331
	2	1305	1185	1085	995	915	840	775	720	670	0.281
	3	1435	1310	1200	1110	1030	955	885	820	765	0.244
	4	1560	1430	1315	1215	1130	1055	990	925	860	0.216
	5	1670	1535	1420	1320	1230	1150	1075	1015	955	0.193
	6	1770	1640	1520	1415	1320	1240	1165	1095	1035	0.175
36/7	0	625	555	495							0.605
	1	800	720	645	585	535					0.456
	2	955	865	785	720	660	610	560	525	490	0.366
	3	1100	1000	915	840	775	720	670	625	585	0.306
	4	1235	1125	1035	955	885	825	770	720	680	0.263
	5	1355	1240	1145	1060	985	920	860	810	765	0.230
	6	1465	1350	1250	1160	1080	1010	950	895	845	0.205
36/5	0	570	510	455							0.726
	1	720	655	595	545	505					0.522
	2	850	780	715	660	610	570	535	500	465	0.408
	3	965	890	820	765	710	665	625	585	555	0.334
	4	1060	985	915	855	800	750	710	670	635	0.283
	5	1145	1070	1000	940	885	835	785	745	705	0.246
	6	1215	1140	1075	1015	960	905	860	815	775	0.217
36/4	0	435	385	345							0.907
	1	580	530	485	445	410					0.610
	2	705	645	595	555	515	480	450	425	395	0.459
	3	800	745	695	645	605	570	535	505	480	0.368
	4	880	825	775	730	685	645	610	580	550	0.307
	5	940	890	845	800	755	715	680	650	615	0.264
	6	995	945	900	855	815	780	740	710	675	0.231

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Grade 80

1.5WR18

Design thickness = 0.0474 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	3.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		5	5.5	6	6.5	7	7.5	8	8.5	9	
36/9	0	1075									0.464
	1	1295	1180	1080							0.381
	2	1485	1365	1265	1170	1080	1005	935	880	825	0.324
	3	1660	1535	1425	1325	1240	1160	1080	1015	955	0.281
	4	1825	1695	1575	1470	1380	1300	1225	1150	1085	0.248
	5	1980	1845	1720	1610	1515	1425	1350	1275	1215	0.223
	6	2125	1985	1855	1740	1640	1550	1465	1390	1320	0.202
36/7	0	660									0.696
	1	890	810	740							0.525
	2	1095	1005	925	855	795	740	690	645	610	0.422
	3	1285	1180	1095	1015	950	890	835	785	740	0.352
	4	1460	1350	1250	1170	1095	1030	970	915	870	0.303
	5	1620	1505	1400	1310	1230	1160	1095	1035	985	0.265
	6	1770	1650	1540	1445	1360	1285	1215	1150	1095	0.236
36/5	0	610									0.835
	1	815	750	690							0.601
	2	990	915	850	795	745	700	660	620	580	0.469
	3	1150	1070	995	935	880	830	785	740	705	0.385
	4	1285	1200	1130	1060	1000	945	900	855	810	0.326
	5	1400	1320	1245	1175	1115	1055	1005	955	915	0.283
	6	1505	1425	1350	1280	1215	1160	1105	1055	1010	0.250
36/4	0	460									1.044
	1	665	615	570							0.702
	2	830	770	720	675	635	595	565	530	500	0.528
	3	970	910	850	800	755	715	680	645	615	0.424
	4	1085	1020	965	915	865	825	785	745	715	0.354
	5	1175	1120	1065	1010	965	920	880	840	805	0.304
	6	1250	1195	1145	1095	1050	1005	965	925	885	0.266

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Grade 80

1.5WR16

Design thickness = 0.0598 in
Support fastening: #12 screws
Side-lap fastening: #10 screws

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.70	2.30
Wind	0.80	3.00
Other	0.70	2.30

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1400									0.428
	2	1645	1530	1425	1325	1235	1160	1090			0.363
	3	1870	1745	1635	1535	1445	1355	1275	1205	1140	0.316
	4	2080	1945	1825	1720	1625	1540	1455	1375	1305	0.279
	5	2275	2135	2010	1895	1795	1700	1615	1540	1470	0.250
	6	2460	2315	2185	2065	1955	1860	1770	1685	1610	0.227
	7	2635	2485	2350	2225	2110	2010	1915	1825	1745	0.207
36/7	1	965									0.590
	2	1220	1135	1060	985	920	865	815			0.474
	3	1455	1355	1265	1190	1120	1060	995	940	895	0.396
	4	1675	1565	1465	1380	1300	1230	1170	1110	1060	0.340
	5	1880	1760	1655	1560	1475	1395	1325	1265	1205	0.298
	6	2070	1945	1830	1730	1640	1555	1480	1410	1350	0.265
	7	2245	2115	2000	1895	1795	1710	1630	1555	1485	0.239
36/5	1	900									0.675
	2	1120	1050	985	925	870	825	780			0.527
	3	1320	1240	1170	1100	1045	990	940	895	855	0.432
	4	1500	1415	1335	1265	1200	1140	1090	1040	995	0.366
	5	1655	1570	1490	1415	1345	1285	1225	1175	1125	0.318
	6	1790	1705	1625	1550	1480	1415	1355	1300	1245	0.281
	7	1905	1825	1745	1670	1600	1535	1475	1415	1360	0.251
36/4	1	740									0.788
	2	950	890	840	790	750	710	675			0.594
	3	1130	1070	1010	955	910	865	825	785	750	0.476
	4	1285	1220	1155	1100	1050	1005	960	920	880	0.397
	5	1405	1345	1285	1230	1175	1125	1080	1040	1000	0.341
	6	1510	1450	1395	1340	1285	1235	1190	1145	1105	0.299
	7	1595	1540	1485	1430	1380	1335	1285	1245	1200	0.266

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

JODEC LLC

HILTI POWER ACTUATED FASTENERS- DIAPHRAGM TABLES

Design thickness = 0.0295 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1520	1335	1190	1055	940					0.352
	1	1615	1425	1275	1145	1025	925	840			0.291
	2	1710	1515	1355	1225	1105	1000	910	835	770	0.248
	3	1800	1600	1435	1300	1185	1075	980	900	830	0.217
	4	1885	1680	1515	1375	1255	1150	1050	965	890	0.192
	5	1965	1760	1590	1445	1320	1215	1120	1025	950	0.172
	6	2040	1835	1660	1515	1385	1280	1185	1090	1010	0.156
36/7	0	970	845	735	650	580					0.528
	1	1085	945	840	740	660	600	545			0.402
	2	1195	1045	930	835	745	675	615	565	520	0.325
	3	1300	1140	1015	915	830	750	685	630	580	0.272
	4	1395	1235	1100	995	905	825	755	695	640	0.235
	5	1490	1320	1185	1070	975	895	825	760	700	0.206
	6	1580	1405	1265	1145	1045	960	885	820	760	0.184
36/5	0	860	755	670	600	535					0.633
	1	955	845	755	685	620	560	510			0.461
	2	1045	930	835	755	690	635	580	530	490	0.362
	3	1125	1010	910	830	760	700	645	595	550	0.298
	4	1200	1085	980	895	825	760	705	655	610	0.253
	5	1265	1150	1050	960	885	820	760	710	665	0.220
	6	1325	1210	1110	1020	940	875	815	760	715	0.195
36/4	0	655	580	515	450	405					0.792
	1	750	665	600	540	485	440	400			0.539
	2	835	750	675	615	560	515	470	430	395	0.409
	3	910	820	745	680	625	575	535	495	455	0.329
	4	975	885	810	745	685	635	590	550	515	0.275
	5	1030	945	870	800	740	690	640	600	565	0.237
	6	1080	995	920	855	795	740	690	650	610	0.208

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1435	1280	1140							0.388
	1	1550	1395	1255	1130	1030					0.321
	2	1655	1500	1365	1235	1125	1030	950	885	820	0.274
	3	1765	1595	1460	1335	1215	1115	1030	955	890	0.239
	4	1865	1695	1550	1425	1310	1205	1110	1030	960	0.211
	5	1965	1785	1635	1510	1395	1290	1190	1105	1030	0.190
	6	2060	1875	1720	1590	1475	1375	1270	1180	1100	0.172
36/7	0	890	785	705							0.581
	1	1025	910	815	735	670					0.443
	2	1145	1030	925	840	765	700	650	605	565	0.358
	3	1260	1135	1030	940	855	790	730	680	635	0.300
	4	1375	1240	1130	1035	950	875	810	750	700	0.258
	5	1480	1340	1220	1120	1035	960	890	825	770	0.227
	6	1585	1435	1315	1210	1115	1040	970	900	840	0.202
36/5	0	810	725	650							0.698
	1	925	835	760	690	625					0.507
	2	1030	935	850	785	720	660	610	570	530	0.399
	3	1130	1025	940	865	805	745	690	640	600	0.328
	4	1220	1115	1025	945	880	820	765	715	670	0.279
	5	1305	1200	1105	1025	950	890	835	785	740	0.243
	6	1385	1275	1180	1095	1020	955	900	845	800	0.215
36/4	0	620	550	490							0.872
	1	730	660	600	545	495					0.594
	2	835	760	695	640	585	540	495	460	430	0.450
	3	925	845	780	720	670	620	575	535	500	0.362
	4	1010	925	855	795	740	690	650	610	570	0.303
	5	1085	1000	930	865	810	755	710	670	635	0.261
	6	1150	1070	995	930	870	820	770	730	690	0.229

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		5	5.5	6	6.5	7	7.5	8	8.5	9	
36/9	0	1510									0.446
	1	1680	1515	1385							0.369
	2	1825	1670	1525	1400	1295	1200	1120	1050	985	0.315
	3	1965	1810	1665	1530	1415	1315	1225	1150	1080	0.274
	4	2105	1935	1795	1660	1535	1430	1335	1250	1175	0.243
	5	2235	2060	1910	1780	1660	1540	1440	1350	1270	0.218
	6	2360	2180	2025	1890	1770	1655	1545	1450	1365	0.198
36/7	0	925									0.669
	1	1095	995	905							0.510
	2	1265	1145	1050	965	890	830	775	725	685	0.412
	3	1415	1295	1190	1095	1015	940	880	825	775	0.345
	4	1560	1430	1320	1225	1135	1055	985	925	870	0.297
	5	1700	1560	1445	1340	1255	1170	1095	1025	965	0.261
	6	1835	1690	1565	1455	1360	1275	1200	1125	1060	0.233
36/5	0	855									0.803
	1	1015	930	850							0.584
	2	1155	1060	980	910	840	780	730	685	645	0.459
	3	1285	1185	1100	1025	960	895	835	785	740	0.378
	4	1410	1305	1215	1135	1060	1000	940	885	835	0.321
	5	1530	1420	1320	1235	1160	1095	1030	980	925	0.279
	6	1635	1525	1425	1335	1255	1185	1120	1060	1010	0.247
36/4	0	650									1.004
	1	810	740	675							0.683
	2	945	870	810	750	690	645	600	560	530	0.518
	3	1070	990	920	860	805	755	705	660	620	0.417
	4	1185	1100	1030	960	905	850	805	760	715	0.349
	5	1290	1205	1125	1060	995	940	890	845	805	0.300
	6	1385	1295	1220	1150	1085	1025	975	925	880	0.263

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1760									0.415
	2	1960	1800	1665	1545	1445	1355	1275			0.354
	3	2145	1985	1835	1710	1595	1495	1405	1325	1255	0.308
	4	2315	2155	2010	1870	1745	1635	1540	1455	1375	0.273
	5	2480	2310	2165	2030	1895	1780	1675	1580	1495	0.245
	6	2640	2465	2310	2170	2045	1920	1810	1705	1615	0.223
	7	2795	2610	2450	2305	2175	2060	1940	1835	1735	0.204
36/7	1	1160									0.573
	2	1360	1250	1160	1075	1005	945	890			0.462
	3	1550	1435	1330	1240	1155	1085	1025	965	915	0.388
	4	1730	1605	1500	1400	1310	1230	1155	1095	1035	0.334
	5	1900	1770	1655	1550	1460	1370	1290	1220	1155	0.293
	6	2070	1930	1805	1695	1595	1510	1425	1345	1275	0.261
	7	2230	2080	1950	1835	1730	1635	1550	1475	1395	0.236
36/5	1	1085									0.656
	2	1265	1175	1095	1020	955	895	840			0.515
	3	1430	1335	1250	1175	1105	1035	975	920	870	0.424
	4	1590	1485	1390	1310	1235	1170	1110	1050	995	0.361
	5	1735	1625	1530	1440	1360	1290	1225	1170	1115	0.314
	6	1875	1760	1660	1565	1485	1410	1340	1275	1220	0.277
	7	2005	1890	1785	1685	1600	1520	1450	1380	1320	0.249
36/4	1	870									0.767
	2	1045	975	910	845	790	740	695			0.582
	3	1205	1125	1055	995	940	880	830	785	740	0.468
	4	1350	1265	1190	1125	1065	1010	960	910	865	0.392
	5	1485	1395	1315	1245	1180	1120	1070	1020	975	0.337
	6	1605	1515	1435	1360	1295	1230	1175	1120	1075	0.296
	7	1720	1630	1545	1470	1400	1335	1275	1220	1165	0.263

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1635	1440	1280	1135	1010					0.211
	1	1735	1530	1365	1225	1095	990	900			0.188
	2	1830	1620	1450	1310	1180	1065	970	890	820	0.169
	3	1920	1705	1530	1385	1260	1140	1040	955	880	0.154
	4	2005	1785	1605	1455	1330	1215	1110	1015	940	0.141
	5	2085	1865	1685	1530	1400	1285	1180	1080	1000	0.130
	6	2165	1940	1755	1600	1465	1350	1250	1145	1060	0.121
36/7	0	1045	910	790	700	625					0.317
	1	1160	1010	895	790	705	640	580			0.267
	2	1270	1110	985	885	790	715	650	600	555	0.230
	3	1375	1210	1075	965	875	790	720	665	615	0.203
	4	1475	1300	1160	1045	950	865	790	725	670	0.181
	5	1570	1390	1245	1125	1025	940	860	790	730	0.163
	6	1660	1475	1325	1200	1095	1005	925	855	790	0.149
36/5	0	925	815	725	645	575					0.380
	1	1025	905	810	730	660	595	545			0.310
	2	1115	990	890	805	735	670	610	560	520	0.262
	3	1195	1070	965	875	800	740	680	625	580	0.227
	4	1270	1145	1035	945	865	800	740	690	640	0.200
	5	1340	1215	1105	1010	930	860	795	745	695	0.179
	6	1400	1275	1170	1070	990	915	850	795	745	0.162
36/4	0	705	620	550	485	435					0.475
	1	805	710	640	575	515	465	425			0.371
	2	890	795	715	650	595	540	495	450	415	0.304
	3	965	870	785	720	660	605	565	515	475	0.258
	4	1030	935	855	780	720	665	620	580	535	0.224
	5	1090	995	915	840	775	720	670	630	590	0.197
	6	1140	1050	970	895	830	775	725	675	635	0.177

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1545	1375	1230							0.233
	1	1660	1495	1340	1210	1105					0.207
	2	1770	1600	1455	1315	1195	1095	1015	940	875	0.186
	3	1875	1700	1550	1415	1290	1185	1090	1015	945	0.169
	4	1980	1795	1640	1510	1380	1270	1170	1090	1015	0.155
	5	2080	1890	1730	1590	1475	1355	1250	1165	1085	0.143
	6	2175	1980	1815	1675	1550	1440	1330	1235	1155	0.133
36/7	0	955	845	755							0.349
	1	1095	970	870	785	715					0.294
	2	1215	1090	980	885	810	745	685	640	595	0.254
	3	1330	1200	1090	990	900	830	765	715	665	0.223
	4	1445	1305	1185	1085	995	915	845	785	735	0.199
	5	1555	1405	1280	1175	1085	1000	925	860	805	0.180
	6	1660	1505	1375	1260	1165	1085	1005	935	875	0.164
36/5	0	875	780	700							0.419
	1	985	890	810	735	670					0.342
	2	1095	990	905	830	760	700	645	600	560	0.289
	3	1195	1085	995	915	845	785	725	675	630	0.250
	4	1285	1175	1080	995	925	860	805	750	700	0.220
	5	1375	1260	1160	1075	1000	930	875	820	770	0.197
	6	1455	1340	1235	1150	1070	1000	940	885	835	0.178
36/4	0	670	590	530							0.523
	1	780	705	640	575	525					0.408
	2	880	800	735	675	620	565	525	485	450	0.335
	3	975	890	820	755	700	655	605	560	520	0.284
	4	1060	975	900	835	775	725	680	635	590	0.246
	5	1140	1050	975	905	845	790	745	700	660	0.217
	6	1210	1120	1045	975	910	855	805	760	720	0.195

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		5	5.5	6	6.5	7	7.5	8	8.5	9	
36/9	0	1625									0.268
	1	1795	1620	1480							0.238
	2	1945	1775	1620	1490	1375	1275	1190	1115	1045	0.214
	3	2085	1915	1760	1620	1495	1390	1295	1215	1140	0.195
	4	2225	2045	1895	1750	1615	1505	1405	1315	1235	0.178
	5	2355	2170	2015	1875	1740	1615	1510	1415	1330	0.165
	6	2485	2295	2130	1985	1855	1730	1615	1515	1425	0.153
36/7	0	1000									0.401
	1	1170	1055	965							0.338
	2	1335	1210	1105	1020	940	875	815	765	720	0.292
	3	1485	1360	1250	1150	1065	990	925	865	815	0.257
	4	1635	1500	1385	1280	1185	1100	1030	965	910	0.229
	5	1775	1630	1510	1400	1305	1215	1135	1065	1005	0.207
	6	1915	1760	1630	1515	1415	1330	1240	1165	1100	0.189
36/5	0	925									0.482
	1	1080	990	900							0.393
	2	1220	1125	1040	960	890	825	770	720	680	0.332
	3	1355	1250	1160	1080	1010	940	875	820	775	0.288
	4	1480	1370	1275	1190	1110	1045	985	920	870	0.254
	5	1600	1485	1385	1295	1215	1140	1075	1020	965	0.227
	6	1715	1595	1490	1395	1310	1235	1165	1105	1050	0.205
36/4	0	700									0.602
	1	860	785	715							0.470
	2	1000	920	850	785	725	675	630	590	555	0.385
	3	1125	1040	965	900	845	790	735	690	650	0.327
	4	1240	1155	1075	1005	945	890	840	790	745	0.283
	5	1350	1260	1175	1105	1040	980	925	880	835	0.250
	6	1445	1355	1270	1195	1130	1065	1010	960	915	0.224

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 45 ksi
Fy: 33 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1880									0.267
	2	2080	1910	1765	1640	1530	1435	1350			0.240
	3	2270	2095	1940	1805	1685	1580	1485	1400	1325	0.219
	4	2440	2270	2110	1965	1835	1720	1620	1525	1445	0.200
	5	2605	2430	2275	2125	1985	1860	1750	1655	1565	0.185
	6	2770	2585	2420	2275	2135	2005	1885	1780	1685	0.172
	7	2925	2735	2565	2410	2275	2145	2020	1905	1805	0.160
36/7	1	1235									0.380
	2	1435	1320	1220	1135	1060	995	935			0.328
	3	1630	1505	1395	1295	1210	1135	1070	1010	960	0.289
	4	1810	1680	1565	1455	1365	1280	1205	1140	1080	0.258
	5	1985	1845	1720	1615	1515	1420	1340	1265	1200	0.233
	6	2150	2005	1875	1760	1655	1565	1475	1390	1320	0.212
	7	2315	2160	2020	1900	1790	1695	1605	1520	1440	0.195
36/5	1	1155									0.442
	2	1335	1240	1155	1075	1005	940	885			0.373
	3	1505	1400	1310	1230	1155	1080	1020	965	910	0.323
	4	1665	1555	1455	1370	1290	1220	1155	1090	1030	0.285
	5	1815	1700	1595	1500	1420	1345	1275	1215	1155	0.255
	6	1955	1835	1730	1630	1540	1465	1390	1325	1265	0.230
	7	2090	1965	1855	1755	1660	1580	1500	1430	1370	0.210
36/4	1	920									0.528
	2	1100	1025	955	885	830	775	730			0.433
	3	1260	1180	1105	1040	980	920	865	815	770	0.367
	4	1410	1320	1240	1170	1105	1050	995	940	890	0.318
	5	1550	1455	1370	1295	1225	1165	1110	1055	1010	0.281
	6	1675	1580	1495	1415	1340	1275	1215	1160	1110	0.252
	7	1790	1695	1605	1525	1450	1380	1320	1260	1205	0.228

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1520	1335	1190	1055	940					0.352
	1	1625	1435	1285	1155	1035	930	850			0.291
	2	1730	1535	1375	1240	1125	1015	925	850	780	0.248
	3	1830	1630	1465	1325	1210	1100	1005	920	850	0.217
	4	1920	1715	1550	1405	1285	1180	1080	990	915	0.192
	5	2005	1800	1630	1480	1360	1250	1155	1060	980	0.172
	6	2090	1885	1705	1555	1430	1320	1225	1135	1050	0.156
36/7	0	970	845	735	650	580					0.528
	1	1100	960	850	750	670	605	555			0.402
	2	1220	1070	950	850	765	690	630	580	535	0.325
	3	1330	1175	1045	940	855	775	710	650	600	0.272
	4	1440	1275	1140	1030	935	860	785	720	670	0.235
	5	1540	1370	1230	1110	1015	930	860	795	735	0.206
	6	1635	1460	1315	1190	1090	1000	925	860	800	0.184
36/5	0	860	755	670	600	535					0.633
	1	965	855	765	690	630	565	515			0.461
	2	1065	950	855	775	705	650	595	545	505	0.362
	3	1150	1035	935	850	780	720	665	615	570	0.298
	4	1230	1115	1010	925	850	785	730	680	635	0.253
	5	1300	1185	1085	995	915	850	790	740	690	0.220
	6	1365	1250	1150	1060	980	910	850	795	745	0.195
36/4	0	655	580	515	450	405					0.792
	1	760	675	605	550	495	445	405			0.539
	2	855	765	690	630	575	530	485	445	410	0.409
	3	930	845	765	700	645	595	555	515	475	0.329
	4	1000	915	835	770	710	660	615	575	540	0.275
	5	1055	975	900	830	770	720	670	630	590	0.237
	6	1105	1025	955	885	825	770	725	680	640	0.208

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1435	1280	1140							0.388
	1	1560	1405	1265	1145	1040					0.321
	2	1680	1520	1385	1255	1145	1050	970	900	840	0.274
	3	1800	1630	1490	1370	1250	1145	1060	980	915	0.239
	4	1910	1735	1590	1460	1350	1240	1145	1065	995	0.211
	5	2015	1835	1685	1555	1440	1335	1235	1145	1070	0.190
	6	2120	1935	1780	1640	1525	1420	1325	1230	1150	0.172
36/7	0	890	785	705							0.581
	1	1035	925	825	750	680					0.443
	2	1170	1050	950	860	785	720	665	620	580	0.358
	3	1300	1170	1065	975	890	815	755	700	655	0.300
	4	1420	1285	1170	1075	990	910	845	785	735	0.258
	5	1540	1395	1275	1170	1080	1005	935	870	810	0.227
	6	1650	1500	1375	1265	1170	1090	1015	950	890	0.202
36/5	0	810	725	650							0.698
	1	935	845	770	700	635					0.507
	2	1050	955	870	800	740	680	630	585	545	0.399
	3	1160	1055	970	895	830	770	720	665	625	0.328
	4	1260	1155	1060	980	910	850	795	750	700	0.279
	5	1350	1240	1150	1065	990	925	870	820	775	0.243
	6	1435	1325	1230	1145	1065	1000	940	885	840	0.215
36/4	0	620	550	490							0.872
	1	745	675	615	555	505					0.594
	2	855	780	715	655	610	560	515	480	445	0.450
	3	955	875	805	745	690	645	605	560	525	0.362
	4	1040	960	890	825	770	720	675	640	600	0.303
	5	1120	1040	965	900	845	790	745	705	665	0.261
	6	1190	1110	1035	970	910	860	810	765	725	0.229

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		5	5.5	6	6.5	7	7.5	8	8.5	9	
36/9	0	1510									0.446
	1	1695	1535	1400							0.369
	2	1860	1705	1555	1430	1320	1225	1145	1070	1005	0.315
	3	2015	1850	1710	1575	1455	1350	1260	1180	1110	0.274
	4	2160	1990	1845	1720	1590	1480	1380	1295	1215	0.243
	5	2305	2130	1975	1840	1725	1605	1500	1405	1320	0.218
	6	2445	2260	2100	1960	1835	1725	1615	1515	1425	0.198
36/7	0	925									0.669
	1	1115	1010	920							0.510
	2	1295	1180	1080	990	920	855	795	750	705	0.412
	3	1460	1340	1235	1140	1055	980	915	860	810	0.345
	4	1620	1490	1375	1280	1190	1105	1035	970	915	0.297
	5	1775	1635	1510	1405	1315	1230	1150	1080	1020	0.261
	6	1920	1770	1645	1530	1430	1345	1265	1190	1125	0.233
36/5	0	855									0.803
	1	1030	945	865							0.584
	2	1185	1090	1010	940	870	805	755	705	665	0.459
	3	1330	1225	1140	1060	995	935	870	820	770	0.378
	4	1465	1355	1265	1180	1105	1040	980	930	875	0.321
	5	1590	1480	1380	1290	1215	1145	1080	1025	975	0.279
	6	1705	1590	1490	1400	1315	1245	1175	1115	1060	0.247
36/4	0	650									1.004
	1	825	755	690							0.683
	2	975	900	835	775	720	670	625	585	550	0.518
	3	1110	1030	960	895	840	790	740	695	655	0.417
	4	1230	1150	1075	1005	945	890	845	800	760	0.349
	5	1340	1255	1180	1110	1045	990	935	890	845	0.300
	6	1440	1355	1275	1205	1140	1080	1025	975	930	0.263

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1780									0.415
	2	2005	1840	1705	1585	1480	1385	1300			0.354
	3	2200	2050	1895	1760	1645	1540	1450	1370	1295	0.308
	4	2390	2225	2080	1940	1810	1700	1600	1510	1430	0.273
	5	2570	2395	2245	2110	1980	1855	1750	1650	1565	0.245
	6	2745	2565	2405	2260	2135	2015	1895	1790	1695	0.223
	7	2910	2725	2555	2410	2275	2155	2045	1935	1830	0.204
36/7	1	1180									0.573
	2	1405	1290	1195	1115	1040	975	920			0.462
	3	1610	1495	1390	1290	1210	1135	1070	1010	955	0.388
	4	1805	1680	1570	1470	1375	1290	1215	1150	1090	0.334
	5	1995	1860	1735	1630	1535	1450	1365	1290	1225	0.293
	6	2175	2030	1900	1785	1685	1595	1510	1430	1355	0.261
	7	2350	2200	2060	1940	1830	1730	1645	1565	1490	0.236

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1635	1440	1280	1135	1010					0.211
	1	1745	1540	1375	1235	1105	995	910			0.188
	2	1850	1640	1465	1325	1200	1080	985	905	835	0.169
	3	1945	1735	1555	1410	1285	1165	1060	975	900	0.154
	4	2040	1825	1640	1490	1360	1250	1140	1045	965	0.141
	5	2130	1910	1725	1565	1435	1320	1215	1115	1030	0.130
	6	2215	1990	1805	1645	1505	1390	1290	1190	1100	0.121
36/7	0	1045	910	790	700	625					0.317
	1	1175	1025	905	800	715	650	590			0.267
	2	1295	1135	1005	905	810	730	665	615	565	0.230
	3	1410	1240	1105	995	900	815	745	685	630	0.203
	4	1520	1340	1200	1080	985	900	820	755	700	0.181
	5	1620	1440	1290	1165	1060	975	900	825	765	0.163
	6	1715	1530	1375	1245	1140	1045	970	900	830	0.149
36/5	0	925	815	725	645	575					0.380
	1	1035	915	815	740	670	605	550			0.310
	2	1130	1010	905	820	750	690	630	575	530	0.262
	3	1220	1095	990	900	825	760	700	650	600	0.227
	4	1300	1175	1065	975	895	825	765	715	665	0.200
	5	1375	1250	1140	1045	960	890	830	775	725	0.179
	6	1440	1315	1205	1110	1025	950	885	830	780	0.162
36/4	0	705	620	550	485	435					0.475
	1	810	720	645	585	525	475	430			0.371
	2	905	810	730	665	610	560	510	465	430	0.304
	3	985	890	810	740	680	625	580	540	495	0.258
	4	1055	965	880	810	745	690	645	600	565	0.224
	5	1115	1025	945	870	810	750	700	655	615	0.197
	6	1170	1085	1000	930	865	805	755	710	665	0.177

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1545	1375	1230							0.233
	1	1670	1505	1355	1225	1115					0.207
	2	1795	1620	1475	1335	1215	1115	1030	955	890	0.186
	3	1910	1730	1580	1450	1320	1210	1120	1040	970	0.169
	4	2025	1835	1680	1545	1425	1305	1210	1120	1045	0.155
	5	2130	1940	1780	1640	1520	1405	1295	1205	1125	0.143
	6	2235	2040	1870	1730	1605	1495	1385	1285	1200	0.133
36/7	0	955	845	755							0.349
	1	1105	985	880	795	725					0.294
	2	1240	1115	1005	910	830	765	705	655	610	0.254
	3	1370	1235	1120	1020	935	860	795	740	690	0.223
	4	1495	1350	1230	1125	1035	955	880	820	765	0.199
	5	1610	1460	1330	1225	1130	1050	970	905	845	0.180
	6	1725	1565	1435	1320	1220	1135	1060	985	920	0.164
36/5	0	875	780	700							0.419
	1	1000	900	820	745	680					0.342
	2	1115	1010	925	850	780	720	665	615	575	0.289
	3	1225	1115	1020	940	875	810	750	700	655	0.250
	4	1325	1215	1115	1030	955	890	835	780	730	0.220
	5	1420	1305	1205	1115	1040	970	910	855	805	0.197
	6	1505	1390	1285	1195	1115	1045	980	925	875	0.178
36/4	0	670	590	530							0.523
	1	790	715	650	590	535					0.408
	2	905	820	755	695	640	585	540	500	470	0.335
	3	1005	920	845	780	725	680	630	585	545	0.284
	4	1095	1010	935	865	805	755	710	665	625	0.246
	5	1180	1090	1015	945	880	825	775	735	695	0.217
	6	1250	1165	1085	1015	950	895	845	795	755	0.195

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		5	5.5	6	6.5	7	7.5	8	8.5	9	
36/9	0	1625									0.268
	1	1815	1640	1495							0.238
	2	1975	1810	1650	1515	1400	1300	1215	1135	1070	0.214
	3	2130	1960	1810	1660	1535	1425	1330	1250	1175	0.195
	4	2285	2105	1945	1810	1670	1555	1450	1360	1280	0.178
	5	2430	2240	2080	1935	1805	1680	1570	1470	1385	0.165
	6	2570	2375	2205	2055	1925	1805	1685	1580	1490	0.153
36/7	0	1000									0.401
	1	1185	1075	980							0.338
	2	1370	1245	1140	1045	970	900	840	790	740	0.292
	3	1535	1410	1295	1190	1105	1025	960	900	845	0.257
	4	1695	1560	1440	1335	1240	1150	1075	1010	950	0.229
	5	1850	1705	1575	1465	1370	1280	1195	1120	1055	0.207
	6	2000	1845	1710	1590	1485	1395	1315	1235	1160	0.189
36/5	0	925									0.482
	1	1095	1005	920							0.393
	2	1250	1150	1065	990	915	850	795	745	700	0.332
	3	1400	1290	1195	1115	1045	975	910	855	805	0.288
	4	1535	1425	1325	1235	1160	1090	1025	965	910	0.254
	5	1665	1545	1440	1350	1265	1195	1125	1070	1015	0.227
	6	1785	1665	1555	1460	1370	1295	1225	1160	1105	0.205
36/4	0	700									0.602
	1	875	800	730							0.470
	2	1025	945	880	815	755	700	655	610	575	0.385
	3	1165	1080	1005	935	880	825	770	725	680	0.327
	4	1290	1200	1120	1050	985	930	880	835	785	0.283
	5	1405	1315	1230	1155	1090	1030	975	925	880	0.250
	6	1510	1415	1330	1255	1185	1120	1065	1010	965	0.224

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 50 ksi
Fy: 40 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1900									0.267
	2	2125	1955	1805	1680	1565	1465	1380			0.240
	3	2330	2160	1995	1855	1735	1625	1530	1440	1365	0.219
	4	2515	2340	2190	2035	1900	1780	1675	1585	1500	0.200
	5	2700	2515	2355	2210	2070	1940	1825	1725	1630	0.185
	6	2875	2685	2515	2365	2230	2095	1975	1865	1765	0.172
	7	3045	2845	2670	2515	2375	2250	2125	2005	1900	0.160
36/7	1	1255									0.380
	2	1480	1360	1260	1170	1095	1025	965			0.328
	3	1690	1565	1450	1350	1260	1185	1115	1055	1000	0.289
	4	1885	1755	1635	1530	1430	1340	1265	1195	1130	0.258
	5	2080	1935	1805	1695	1595	1500	1415	1335	1265	0.233
	6	2260	2110	1975	1855	1745	1650	1560	1475	1400	0.212
	7	2440	2280	2135	2005	1895	1790	1700	1615	1535	0.195
36/5	1	1180									0.442
	2	1375	1280	1195	1110	1035	970	915			0.373
	3	1555	1450	1360	1275	1205	1130	1065	1005	950	0.323
	4	1730	1620	1520	1430	1350	1275	1210	1145	1085	0.285
	5	1895	1775	1670	1575	1490	1410	1340	1275	1220	0.255
	6	2050	1925	1815	1715	1620	1540	1465	1395	1335	0.230
	7	2190	2065	1950	1845	1750	1665	1585	1515	1445	0.210
36/4	1	945									0.528
	2	1140	1060	995	925	860	810	760			0.433
	3	1310	1225	1150	1085	1025	965	910	860	810	0.367
	4	1475	1380	1300	1225	1160	1100	1045	1000	945	0.318
	5	1620	1525	1440	1360	1290	1230	1170	1115	1065	0.281
	6	1755	1655	1570	1490	1415	1345	1285	1230	1175	0.252
	7	1875	1780	1690	1605	1530	1460	1395	1335	1280	0.228

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1520	1335	1190	1055	940					0.352
	1	1650	1455	1300	1175	1050	950	865			0.291
	2	1770	1575	1410	1275	1160	1050	955	875	810	0.248
	3	1885	1680	1515	1375	1255	1150	1050	965	890	0.217
	4	1990	1785	1615	1465	1345	1235	1140	1050	970	0.192
	5	2090	1885	1705	1555	1430	1320	1225	1135	1050	0.172
	6	2180	1975	1795	1645	1510	1395	1300	1210	1125	0.156
36/7	0	970	845	735	650	580					0.528
	1	1120	980	870	770	690	625	570			0.402
	2	1265	1110	990	890	800	725	660	610	560	0.325
	3	1395	1235	1100	995	905	825	755	695	640	0.272
	4	1520	1350	1210	1095	1000	915	845	780	720	0.235
	5	1635	1460	1315	1190	1090	1000	925	860	800	0.206
	6	1735	1560	1410	1285	1180	1085	1005	935	875	0.184
36/5	0	860	755	670	600	535					0.633
	1	985	875	785	710	645	585	530			0.461
	2	1100	985	885	805	735	675	625	575	530	0.362
	3	1200	1085	980	895	825	760	705	655	610	0.298
	4	1290	1170	1070	980	905	835	780	725	680	0.253
	5	1365	1250	1150	1060	980	910	850	795	745	0.220
	6	1430	1320	1220	1130	1050	980	915	860	805	0.195
36/4	0	655	580	515	450	405					0.792
	1	780	695	625	565	515	465	420			0.539
	2	885	795	720	660	605	555	515	470	435	0.409
	3	975	885	810	745	685	635	590	550	515	0.329
	4	1045	960	885	820	760	705	660	615	580	0.275
	5	1105	1025	955	885	825	770	725	680	640	0.237
	6	1155	1080	1010	945	885	830	785	740	695	0.208

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1435	1280	1140							0.388
	1	1585	1430	1290	1165	1060					0.321
	2	1730	1565	1425	1300	1185	1090	1005	930	870	0.274
	3	1865	1695	1550	1425	1310	1205	1110	1030	960	0.239
	4	1995	1815	1665	1535	1420	1315	1215	1130	1055	0.211
	5	2120	1935	1780	1640	1525	1420	1325	1230	1150	0.190
	6	2235	2050	1885	1745	1625	1515	1420	1330	1240	0.172
36/7	0	890	785	705							0.581
	1	1065	950	850	770	700					0.443
	2	1220	1100	1000	905	825	760	700	655	610	0.358
	3	1375	1240	1130	1035	950	875	810	750	700	0.300
	4	1515	1375	1255	1150	1065	990	915	850	795	0.258
	5	1650	1500	1375	1265	1170	1090	1015	950	890	0.227
	6	1775	1620	1485	1375	1275	1185	1110	1040	980	0.202
36/5	0	810	725	650							0.698
	1	960	865	790	720	660					0.507
	2	1095	995	910	840	775	720	665	615	575	0.399
	3	1220	1115	1025	945	880	820	765	715	670	0.328
	4	1330	1225	1130	1050	975	910	855	805	760	0.279
	5	1435	1325	1230	1145	1065	1000	940	885	840	0.243
	6	1525	1415	1320	1230	1155	1085	1020	965	910	0.215
36/4	0	620	550	490							0.872
	1	765	695	635	575	525					0.594
	2	895	815	750	695	645	595	550	510	475	0.450
	3	1010	925	855	795	740	690	650	610	570	0.362
	4	1105	1025	950	885	830	780	730	690	655	0.303
	5	1190	1110	1035	970	910	860	810	765	725	0.261
	6	1260	1185	1115	1050	990	935	885	835	795	0.229

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		5	5.5	6	6.5	7	7.5	8	8.5	9	
36/9	0	1510									0.446
	1	1730	1570	1430							0.369
	2	1920	1765	1620	1485	1375	1275	1190	1115	1050	0.315
	3	2105	1935	1795	1660	1535	1430	1335	1250	1175	0.274
	4	2275	2100	1950	1815	1700	1580	1475	1380	1300	0.243
	5	2445	2260	2100	1960	1835	1725	1615	1515	1425	0.218
	6	2600	2415	2245	2100	1970	1855	1750	1650	1550	0.198
36/7	0	925									0.669
	1	1155	1045	955							0.510
	2	1365	1250	1140	1050	970	905	845	790	745	0.412
	3	1560	1430	1320	1225	1135	1055	985	925	870	0.345
	4	1745	1605	1485	1380	1290	1205	1130	1060	995	0.297
	5	1920	1770	1645	1530	1430	1345	1265	1190	1125	0.261
	6	2090	1930	1795	1675	1570	1475	1390	1315	1250	0.233
36/5	0	855									0.803
	1	1060	975	895							0.584
	2	1245	1145	1060	990	920	860	800	750	705	0.459
	3	1410	1305	1215	1135	1060	1000	940	885	835	0.378
	4	1565	1455	1355	1270	1195	1125	1060	1005	955	0.321
	5	1705	1590	1490	1400	1315	1245	1175	1115	1060	0.279
	6	1830	1720	1615	1520	1435	1355	1285	1225	1165	0.247
36/4	0	650									1.004
	1	855	785	720							0.683
	2	1030	950	885	825	770	720	670	630	590	0.518
	3	1185	1100	1030	960	905	850	805	760	715	0.417
	4	1320	1235	1160	1090	1025	970	920	870	830	0.349
	5	1440	1355	1275	1205	1140	1080	1025	975	930	0.300
	6	1545	1460	1380	1310	1245	1180	1125	1075	1025	0.263

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1825									0.415
	2	2090	1925	1780	1655	1545	1450	1360			0.354
	3	2315	2155	2010	1870	1745	1635	1540	1455	1375	0.308
	4	2535	2360	2210	2080	1945	1825	1720	1625	1535	0.273
	5	2745	2565	2405	2260	2135	2015	1895	1790	1695	0.245
	6	2945	2755	2590	2440	2305	2180	2070	1960	1860	0.223
	7	3135	2940	2765	2610	2470	2340	2225	2120	2020	0.204
36/7	1	1225									0.573
	2	1490	1375	1275	1185	1105	1040	980			0.462
	3	1730	1605	1500	1400	1310	1230	1155	1095	1035	0.388
	4	1960	1825	1705	1600	1505	1415	1335	1260	1195	0.334
	5	2175	2030	1900	1785	1685	1595	1510	1430	1355	0.293
	6	2385	2230	2090	1970	1860	1760	1670	1590	1515	0.261
	7	2585	2420	2275	2145	2025	1920	1825	1735	1655	0.236
36/5	1	1150									0.656
	2	1375	1280	1200	1125	1055	990	930			0.515
	3	1590	1485	1390	1310	1235	1170	1110	1050	995	0.424
	4	1785	1670	1575	1485	1405	1330	1265	1205	1150	0.361
	5	1965	1850	1745	1650	1560	1485	1410	1345	1285	0.314
	6	2130	2010	1900	1800	1710	1630	1555	1485	1420	0.277
	7	2280	2160	2050	1945	1855	1765	1685	1615	1545	0.249
36/4	1	935									0.767
	2	1155	1075	1010	950	890	835	785			0.582
	3	1350	1265	1190	1125	1065	1010	960	910	865	0.468
	4	1525	1440	1360	1285	1220	1160	1105	1055	1005	0.392
	5	1680	1590	1510	1435	1365	1300	1240	1185	1135	0.337
	6	1820	1730	1645	1570	1495	1430	1370	1310	1260	0.296
	7	1940	1850	1770	1690	1620	1550	1485	1430	1370	0.263

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1635	1440	1280	1135	1010					0.211
	1	1765	1560	1395	1255	1125	1015	925			0.188
	2	1890	1675	1505	1360	1235	1115	1015	930	860	0.169
	3	2005	1785	1605	1455	1330	1215	1110	1015	940	0.154
	4	2110	1890	1705	1550	1420	1305	1200	1105	1020	0.141
	5	2215	1990	1805	1645	1505	1390	1290	1190	1100	0.130
	6	2305	2085	1895	1730	1590	1470	1365	1270	1175	0.121
36/7	0	1045	910	790	700	625					0.317
	1	1195	1045	925	820	735	665	605			0.267
	2	1340	1175	1045	940	845	765	700	640	595	0.230
	3	1475	1300	1160	1045	950	865	790	725	670	0.203
	4	1600	1420	1270	1150	1045	960	885	815	750	0.181
	5	1715	1530	1375	1245	1140	1045	970	900	830	0.163
	6	1825	1635	1475	1340	1230	1130	1045	975	910	0.149
36/5	0	925	815	725	645	575					0.380
	1	1055	935	835	755	685	620	565			0.310
	2	1170	1045	940	855	780	715	660	605	560	0.262
	3	1270	1145	1035	945	865	800	740	690	640	0.227
	4	1360	1235	1125	1030	950	880	815	760	715	0.200
	5	1440	1315	1205	1110	1025	950	885	830	780	0.179
	6	1510	1390	1280	1185	1100	1025	955	895	840	0.162
36/4	0	705	620	550	485	435					0.475
	1	830	740	665	600	545	490	445			0.371
	2	940	845	765	695	635	585	540	495	455	0.304
	3	1030	935	855	780	720	665	620	580	535	0.258
	4	1105	1015	930	860	795	740	690	645	605	0.224
	5	1170	1085	1000	930	865	805	755	710	665	0.197
	6	1225	1140	1065	995	930	870	815	770	725	0.177

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1545	1375	1230							0.233
	1	1695	1530	1380	1245	1135					0.207
	2	1840	1665	1520	1380	1260	1155	1065	990	920	0.186
	3	1980	1795	1640	1510	1380	1270	1170	1090	1015	0.169
	4	2110	1920	1760	1620	1500	1385	1280	1185	1110	0.155
	5	2235	2040	1870	1730	1605	1495	1385	1285	1200	0.143
	6	2355	2155	1980	1835	1705	1590	1490	1385	1295	0.133
36/7	0	955	845	755							0.349
	1	1135	1010	905	820	745					0.294
	2	1290	1160	1055	955	870	800	740	690	640	0.254
	3	1445	1305	1185	1085	995	915	845	785	735	0.223
	4	1590	1440	1310	1205	1110	1030	955	885	830	0.199
	5	1725	1565	1435	1320	1220	1135	1060	985	920	0.180
	6	1855	1690	1550	1430	1325	1235	1155	1080	1015	0.164
36/5	0	875	780	700							0.419
	1	1020	925	840	765	700					0.342
	2	1160	1055	965	885	820	755	700	650	605	0.289
	3	1285	1175	1080	995	925	860	805	750	700	0.250
	4	1400	1285	1185	1100	1020	955	895	840	790	0.220
	5	1505	1390	1285	1195	1115	1045	980	925	875	0.197
	6	1600	1485	1380	1285	1205	1130	1065	1005	950	0.178
36/4	0	670	590	530							0.523
	1	815	740	675	610	555					0.408
	2	945	860	790	730	675	625	575	535	500	0.335
	3	1060	975	900	835	775	725	680	635	590	0.284
	4	1165	1075	995	930	865	810	765	720	680	0.246
	5	1250	1165	1085	1015	950	895	845	795	755	0.217
	6	1325	1245	1165	1095	1030	970	920	870	825	0.195

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		5	5.5	6	6.5	7	7.5	8	8.5	9	
36/9	0	1625									0.268
	1	1850	1675	1525							0.238
	2	2040	1875	1715	1575	1455	1350	1260	1180	1110	0.214
	3	2225	2045	1895	1750	1615	1505	1405	1315	1235	0.195
	4	2400	2215	2050	1910	1780	1655	1545	1450	1360	0.178
	5	2570	2375	2205	2055	1925	1805	1685	1580	1490	0.165
	6	2730	2530	2355	2200	2060	1940	1830	1715	1615	0.153
36/7	0	1000									0.401
	1	1225	1110	1010							0.338
	2	1440	1315	1200	1105	1020	950	890	835	785	0.292
	3	1635	1500	1385	1280	1185	1100	1030	965	910	0.257
	4	1820	1675	1550	1440	1345	1255	1170	1100	1035	0.229
	5	2000	1845	1710	1590	1485	1395	1315	1235	1160	0.207
	6	2170	2005	1865	1740	1625	1530	1440	1360	1290	0.189
36/5	0	925									0.482
	1	1130	1035	950							0.393
	2	1310	1210	1120	1040	970	900	840	790	740	0.332
	3	1480	1370	1275	1190	1110	1045	985	920	870	0.288
	4	1640	1520	1420	1325	1245	1175	1105	1050	995	0.254
	5	1785	1665	1555	1460	1370	1295	1225	1160	1105	0.227
	6	1915	1795	1680	1580	1490	1410	1335	1270	1210	0.205
36/4	0	700									0.602
	1	910	835	760							0.470
	2	1085	1000	930	865	810	750	700	655	615	0.385
	3	1240	1155	1075	1005	945	890	840	790	745	0.327
	4	1385	1290	1210	1135	1070	1010	955	905	860	0.283
	5	1510	1415	1330	1255	1185	1120	1065	1010	965	0.250
	6	1615	1525	1440	1365	1290	1225	1165	1110	1060	0.224

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 60 ksi
Fy: 50 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1945									0.267
	2	2215	2035	1880	1750	1635	1530	1440			0.240
	3	2440	2270	2110	1965	1835	1720	1620	1525	1445	0.219
	4	2660	2480	2320	2175	2035	1910	1795	1695	1605	0.200
	5	2875	2685	2515	2365	2230	2095	1975	1865	1765	0.185
	6	3080	2880	2705	2545	2405	2275	2155	2035	1925	0.172
	7	3275	3070	2885	2720	2570	2435	2315	2205	2085	0.160
36/7	1	1300									0.380
	2	1570	1445	1335	1245	1160	1090	1025			0.328
	3	1810	1680	1565	1455	1365	1280	1205	1140	1080	0.289
	4	2040	1900	1775	1665	1565	1470	1385	1310	1240	0.258
	5	2260	2110	1975	1855	1745	1650	1560	1475	1400	0.233
	6	2475	2310	2165	2035	1920	1820	1725	1640	1560	0.212
	7	2680	2505	2350	2215	2095	1980	1880	1790	1710	0.195
36/5	1	1220									0.442
	2	1450	1350	1260	1180	1105	1035	975			0.373
	3	1665	1555	1455	1370	1290	1220	1155	1090	1030	0.323
	4	1860	1745	1640	1545	1460	1385	1315	1250	1195	0.285
	5	2050	1925	1815	1715	1620	1540	1465	1395	1335	0.255
	6	2220	2090	1975	1870	1775	1690	1610	1535	1470	0.230
	7	2375	2245	2130	2020	1920	1830	1745	1670	1600	0.210
36/4	1	990									0.528
	2	1210	1130	1055	995	930	870	820			0.433
	3	1410	1320	1240	1170	1105	1050	995	940	890	0.367
	4	1590	1500	1415	1335	1265	1205	1145	1090	1045	0.318
	5	1755	1655	1570	1490	1415	1345	1285	1230	1175	0.281
	6	1900	1800	1710	1630	1550	1480	1415	1355	1300	0.252
	7	2025	1930	1840	1755	1680	1605	1540	1475	1415	0.228

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0295 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1520	1335	1190	1055	940					0.352
	1	1655	1460	1305	1180	1055	950	865			0.291
	2	1780	1580	1415	1280	1170	1055	960	885	815	0.248
	3	1895	1695	1525	1385	1265	1160	1060	970	895	0.217
	4	2005	1800	1625	1480	1355	1250	1155	1060	980	0.192
	5	2105	1900	1720	1570	1445	1330	1235	1150	1060	0.172
	6	2195	1990	1815	1660	1530	1415	1310	1225	1145	0.156
36/7	0	970	845	735	650	580					0.528
	1	1125	985	875	775	695	625	570			0.402
	2	1275	1120	995	895	810	730	670	615	565	0.325
	3	1410	1245	1115	1005	915	835	765	700	650	0.272
	4	1535	1365	1225	1110	1010	930	855	790	730	0.235
	5	1650	1475	1330	1210	1105	1015	940	875	815	0.206
	6	1755	1580	1430	1305	1195	1100	1020	950	890	0.184
36/5	0	860	755	670	600	535					0.633
	1	990	880	785	710	650	585	535			0.461
	2	1110	990	895	810	740	685	630	580	535	0.362
	3	1210	1090	990	905	830	765	710	665	615	0.298
	4	1300	1180	1080	990	915	845	790	735	690	0.253
	5	1375	1260	1160	1070	990	920	860	805	755	0.220
	6	1440	1335	1235	1145	1065	990	930	870	820	0.195
36/4	0	655	580	515	450	405					0.792
	1	785	700	625	570	520	465	425			0.539
	2	890	805	730	665	610	560	520	480	440	0.409
	3	980	895	820	750	695	640	595	560	525	0.329
	4	1055	970	895	830	770	715	670	625	590	0.275
	5	1115	1035	965	895	835	780	735	690	650	0.237
	6	1165	1090	1025	960	900	845	795	750	710	0.208

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0358 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1435	1280	1140							0.388
	1	1590	1435	1295	1170	1065					0.321
	2	1740	1575	1435	1310	1195	1095	1010	940	875	0.274
	3	1880	1705	1560	1435	1320	1215	1120	1040	970	0.239
	4	2010	1835	1680	1550	1435	1335	1230	1145	1065	0.211
	5	2140	1955	1795	1660	1540	1435	1340	1245	1165	0.190
	6	2255	2070	1905	1765	1640	1535	1435	1350	1260	0.172
36/7	0	890	785	705							0.581
	1	1070	955	855	775	705					0.443
	2	1230	1110	1005	915	835	765	710	660	615	0.358
	3	1385	1255	1140	1045	965	885	820	760	710	0.300
	4	1535	1390	1270	1165	1080	1000	930	865	810	0.258
	5	1670	1520	1390	1280	1185	1105	1035	965	905	0.227
	6	1800	1645	1510	1395	1295	1205	1130	1060	1000	0.202
36/5	0	810	725	650							0.698
	1	965	870	795	725	660					0.507
	2	1105	1005	920	845	785	725	670	625	580	0.399
	3	1230	1125	1035	955	890	830	775	725	680	0.328
	4	1345	1240	1145	1060	990	925	865	815	770	0.279
	5	1450	1340	1245	1160	1080	1015	955	900	850	0.243
	6	1540	1435	1335	1250	1170	1100	1035	980	925	0.215
36/4	0	620	550	490							0.872
	1	770	700	640	580	530					0.594
	2	905	825	760	700	650	605	560	520	485	0.450
	3	1020	940	865	805	750	700	655	620	580	0.362
	4	1120	1035	965	900	840	790	745	700	665	0.303
	5	1205	1125	1050	985	925	870	825	780	740	0.261
	6	1275	1200	1130	1060	1000	945	895	850	810	0.229

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0474 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		5	5.5	6	6.5	7	7.5	8	8.5	9	
36/9	0	1510									0.446
	1	1735	1575	1435							0.369
	2	1935	1775	1630	1500	1385	1285	1200	1125	1055	0.315
	3	2120	1955	1810	1680	1555	1445	1345	1260	1185	0.274
	4	2300	2125	1970	1835	1720	1600	1495	1400	1315	0.243
	5	2470	2285	2125	1985	1860	1750	1640	1540	1445	0.218
	6	2635	2445	2275	2130	1995	1880	1775	1675	1575	0.198
36/7	0	925									0.669
	1	1160	1050	960							0.510
	2	1375	1260	1155	1060	985	915	855	800	755	0.412
	3	1575	1445	1335	1240	1150	1070	1000	940	885	0.345
	4	1770	1625	1505	1400	1310	1225	1145	1075	1015	0.297
	5	1950	1800	1670	1555	1455	1365	1285	1215	1145	0.261
	6	2120	1960	1825	1705	1595	1500	1415	1340	1270	0.233
36/5	0	855									0.803
	1	1070	980	900							0.584
	2	1255	1155	1070	1000	935	870	810	760	715	0.459
	3	1425	1320	1230	1145	1075	1010	955	900	845	0.378
	4	1585	1475	1375	1290	1210	1140	1080	1020	970	0.321
	5	1725	1615	1510	1420	1335	1265	1195	1135	1080	0.279
	6	1855	1740	1635	1545	1455	1380	1310	1245	1185	0.247
36/4	0	650									1.004
	1	860	795	730							0.683
	2	1040	960	895	835	780	730	680	635	600	0.518
	3	1200	1115	1040	975	915	865	815	775	730	0.417
	4	1340	1250	1175	1105	1040	985	935	885	845	0.349
	5	1460	1375	1295	1225	1155	1095	1040	990	945	0.300
	6	1565	1480	1400	1330	1260	1200	1145	1090	1045	0.263

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: Hilti X-HSN24
Side-lap fastening: #10 screws
0.125" through 0.375" Support Steel

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1835									0.415
	2	2105	1940	1795	1670	1560	1460	1375			0.354
	3	2335	2175	2030	1890	1765	1655	1560	1470	1390	0.308
	4	2560	2390	2240	2105	1975	1850	1740	1645	1560	0.273
	5	2775	2595	2435	2290	2160	2045	1925	1820	1725	0.245
	6	2985	2795	2625	2475	2335	2215	2105	1995	1890	0.223
	7	3180	2985	2805	2650	2505	2375	2260	2155	2055	0.204
36/7	1	1235									0.573
	2	1510	1390	1290	1200	1120	1050	990			0.462
	3	1755	1630	1520	1420	1330	1245	1175	1110	1050	0.388
	4	1990	1850	1730	1625	1530	1440	1360	1285	1220	0.334
	5	2215	2065	1935	1815	1715	1620	1535	1460	1385	0.293
	6	2425	2270	2130	2005	1890	1790	1700	1620	1545	0.261
	7	2630	2465	2315	2185	2065	1955	1860	1770	1690	0.236
36/5	1	1160									0.656
	2	1390	1295	1210	1140	1065	1000	940			0.515
	3	1610	1505	1410	1325	1255	1185	1125	1065	1010	0.424
	4	1810	1695	1595	1505	1425	1350	1285	1225	1170	0.361
	5	1990	1875	1770	1675	1585	1510	1435	1370	1310	0.314
	6	2160	2040	1930	1830	1740	1655	1580	1510	1445	0.277
	7	2310	2190	2080	1980	1885	1800	1715	1645	1575	0.249
36/4	1	940									0.767
	2	1165	1090	1020	960	905	850	800			0.582
	3	1370	1285	1210	1140	1080	1025	975	925	880	0.468
	4	1550	1460	1380	1305	1240	1180	1125	1070	1025	0.392
	5	1705	1615	1535	1455	1385	1320	1265	1210	1155	0.337
	6	1845	1755	1670	1595	1520	1455	1395	1335	1280	0.296
	7	1965	1875	1795	1715	1645	1575	1515	1455	1400	0.263

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Grade 80

1.5WR22

Design thickness = 0.0295 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		3	3.5	4	4.5	5	5.5	6	6.5	7	
36/9	0	1635	1440	1280	1135	1010					0.211
	1	1770	1565	1395	1260	1125	1020	925			0.188
	2	1895	1685	1510	1365	1240	1120	1020	935	865	0.169
	3	2015	1800	1615	1465	1340	1225	1120	1025	945	0.154
	4	2125	1905	1720	1565	1430	1320	1215	1115	1030	0.141
	5	2230	2005	1820	1660	1520	1405	1300	1205	1110	0.130
	6	2325	2105	1910	1750	1605	1485	1380	1285	1195	0.121
36/7	0	1045	910	790	700	625					0.317
	1	1200	1050	930	825	740	670	610			0.267
	2	1350	1185	1055	945	855	770	705	645	600	0.230
	3	1490	1315	1170	1055	960	875	800	735	680	0.203
	4	1615	1435	1285	1160	1060	970	895	825	760	0.181
	5	1735	1550	1395	1265	1155	1060	980	910	845	0.163
	6	1845	1655	1495	1360	1245	1150	1065	990	925	0.149
36/5	0	925	815	725	645	575					0.380
	1	1060	935	840	760	690	625	570			0.310
	2	1175	1050	945	860	785	720	665	610	565	0.262
	3	1280	1155	1045	955	875	805	750	695	645	0.227
	4	1375	1245	1135	1040	960	890	825	770	720	0.200
	5	1455	1330	1220	1125	1040	965	900	840	790	0.179
	6	1525	1405	1295	1200	1115	1035	970	910	855	0.162
36/4	0	705	620	550	485	435					0.475
	1	835	745	670	605	550	495	450			0.371
	2	945	850	770	700	645	595	545	500	460	0.304
	3	1040	945	860	790	730	675	625	585	545	0.258
	4	1115	1025	940	870	805	750	700	655	615	0.224
	5	1180	1095	1015	940	875	820	765	720	675	0.197
	6	1235	1150	1075	1005	940	880	830	780	735	0.177

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		3	3.5	4	4.5	5	5.5	6	6.5	7
WR	0.173	15257	11209	8582	6781	5493	4539	3814	3250	2802

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Grade 80

1.5WR20

Design thickness = 0.0358 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		4	4.5	5	5.5	6	6.5	7	7.5	8	
36/9	0	1545	1375	1230							0.233
	1	1700	1535	1385	1250	1140					0.207
	2	1850	1675	1525	1390	1265	1160	1075	995	930	0.186
	3	1990	1810	1650	1520	1395	1280	1185	1100	1025	0.169
	4	2125	1935	1775	1635	1515	1400	1295	1200	1120	0.155
	5	2255	2060	1890	1745	1620	1510	1405	1305	1215	0.143
	6	2375	2175	2005	1855	1720	1605	1505	1405	1310	0.133
36/7	0	955	845	755							0.349
	1	1140	1015	910	825	750					0.294
	2	1305	1170	1065	965	880	810	745	695	650	0.254
	3	1460	1315	1200	1100	1010	925	855	795	745	0.223
	4	1610	1455	1330	1220	1125	1045	965	900	840	0.199
	5	1750	1590	1455	1335	1235	1150	1075	1000	935	0.180
	6	1880	1715	1570	1450	1345	1250	1170	1100	1035	0.164
36/5	0	875	780	700							0.419
	1	1025	930	845	770	705					0.342
	2	1170	1060	970	895	830	765	705	655	610	0.289
	3	1300	1185	1090	1005	935	870	815	760	710	0.250
	4	1415	1300	1200	1110	1035	965	905	855	805	0.220
	5	1525	1405	1305	1210	1130	1060	995	935	885	0.197
	6	1620	1500	1395	1305	1220	1145	1080	1020	965	0.178
36/4	0	670	590	530							0.523
	1	820	740	680	615	560					0.408
	2	955	870	800	735	685	630	585	540	505	0.335
	3	1070	985	910	845	785	735	685	645	600	0.284
	4	1175	1090	1010	940	880	825	775	730	690	0.246
	5	1265	1180	1100	1030	965	910	855	810	765	0.217
	6	1340	1260	1180	1110	1045	985	930	885	840	0.195

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		4	4.5	5	5.5	6	6.5	7	7.5	8
WR	0.210	11482	9072	7348	6073	5103	4348	3749	3266	2870

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Grade 80

1.5WR18

Design thickness = 0.0474 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft. 5	5.5	6	6.5	7	7.5	8	8.5	9	K_1 1/ft
36/9	0	1625									0.268
	1	1855	1680	1530							0.238
	2	2050	1885	1725	1585	1465	1360	1270	1190	1120	0.214
	3	2240	2065	1910	1765	1635	1520	1415	1330	1250	0.195
	4	2425	2235	2075	1930	1800	1675	1565	1465	1380	0.178
	5	2595	2400	2230	2080	1950	1830	1710	1605	1510	0.165
	6	2760	2560	2380	2225	2085	1965	1855	1740	1640	0.153
36/7	0	1000									0.401
	1	1230	1115	1020							0.338
	2	1450	1330	1215	1115	1035	960	900	840	790	0.292
	3	1655	1515	1400	1295	1200	1115	1045	980	920	0.257
	4	1845	1700	1570	1460	1365	1275	1190	1115	1050	0.229
	5	2030	1870	1735	1615	1510	1420	1335	1255	1185	0.207
	6	2205	2040	1895	1765	1655	1555	1465	1385	1315	0.189
36/5	0	925									0.482
	1	1135	1040	955							0.393
	2	1325	1220	1130	1050	980	910	850	800	750	0.332
	3	1500	1385	1290	1200	1125	1060	995	935	880	0.288
	4	1660	1540	1435	1345	1265	1190	1125	1065	1010	0.254
	5	1805	1685	1575	1480	1395	1315	1245	1180	1120	0.227
	6	1940	1820	1705	1605	1515	1435	1360	1290	1230	0.205
36/4	0	700									0.602
	1	915	840	770							0.470
	2	1095	1010	940	875	820	760	710	665	625	0.385
	3	1255	1170	1090	1020	955	900	850	805	755	0.327
	4	1400	1310	1225	1150	1085	1025	970	920	875	0.283
	5	1525	1435	1350	1275	1205	1140	1080	1030	980	0.250
	6	1640	1545	1460	1385	1310	1245	1185	1130	1080	0.224

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in^4/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft 5	5.5	6	6.5	7	7.5	8	8.5	9
WR	0.279	11211	9265	7786	6634	5720	4983	4379	3879	3460

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\ \Omega_{df}, S_{nb}/\ \Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}$, $\varphi_{db}S_{nb}$}

Design thickness = 0.0598 in
Support fastening: Hilti X-ENP-19 L5
Side-lap fastening: #10 screws
0.250" and thicker Support Steel

Fu: 62 ksi
Fy: 60 ksi

Loading	φ_{df}	Ω_{df}
Seismic	0.65	2.50
Wind	0.70	2.35
Other	0.65	2.50

Fastener Layout	Side-lap Conn/ Span	NOMINAL SHEAR STRENGTH, S_{nf}, plf[1,2] Span, ft.									K_1 1/ft
		6	6.5	7	7.5	8	8.5	9	9.5	10	
36/9	1	1955									0.267
	2	2230	2050	1895	1765	1645	1545	1450			0.240
	3	2465	2295	2135	1985	1855	1740	1635	1545	1460	0.219
	4	2690	2510	2350	2205	2060	1935	1820	1720	1625	0.200
	5	2910	2715	2545	2395	2260	2130	2005	1895	1795	0.185
	6	3120	2920	2740	2580	2435	2310	2190	2070	1960	0.172
	7	3320	3110	2925	2760	2610	2475	2350	2240	2125	0.160
36/7	1	1310									0.380
	2	1585	1460	1350	1255	1175	1105	1040			0.328
	3	1835	1700	1590	1480	1385	1300	1225	1155	1095	0.289
	4	2070	1925	1800	1690	1590	1495	1405	1330	1260	0.258
	5	2300	2145	2005	1885	1775	1680	1590	1505	1425	0.233
	6	2515	2350	2205	2075	1955	1850	1755	1670	1595	0.212
	7	2725	2550	2395	2255	2130	2020	1920	1825	1740	0.195
36/5	1	1230									0.442
	2	1465	1365	1275	1195	1115	1050	985			0.373
	3	1685	1570	1475	1385	1310	1240	1170	1105	1050	0.323
	4	1890	1770	1665	1570	1485	1405	1335	1270	1215	0.285
	5	2075	1955	1840	1740	1650	1565	1490	1420	1355	0.255
	6	2250	2125	2005	1900	1805	1715	1635	1565	1495	0.230
	7	2410	2280	2160	2055	1955	1860	1775	1700	1625	0.210
36/4	1	995									0.528
	2	1225	1140	1070	1005	940	885	830			0.433
	3	1430	1340	1260	1190	1125	1065	1010	960	910	0.367
	4	1615	1520	1435	1355	1285	1225	1165	1110	1060	0.318
	5	1780	1680	1595	1515	1440	1370	1305	1250	1195	0.281
	6	1925	1830	1740	1655	1580	1505	1440	1380	1325	0.252
	7	2055	1960	1870	1785	1705	1635	1565	1505	1445	0.228

Notes: 1. Nominal shear strength shown above may be limited by shear buckling. See Table below.

Buckling:	φ_{db}	Ω_{db}
	0.80	2.00

Deck Profile	I in⁴/ft.	NOMINAL SHEAR DUE TO PANEL BUCKLING, S_{nb}, plf[2] Span, ft								
		6	6.5	7	7.5	8	8.5	9	9.5	10
WR	0.353	11049	9414	8117	7071	6215	5505	4910	4407	3977

Notes: 2. Design Strengths:

ASD Required strength (Service Applied Load) <= Min {$S_{nf}/\Omega_{df}, S_{nb}/\Omega_{db}$}

LRFD Required strength (Factored Applied Load) <= Min {$\varphi_{df}S_{nf}, \varphi_{db}S_{nb}$}